# Development And Design of Royal City 

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#### Abstract

In old time stone bricks were popular building material nowadays cement concrete is conveniently used for construction of multistoried construction reinforced cement concrete (R.C.C) is used .r.c.c. is composite structure made up of concrete and steel.reinforcement is used to carry tensile stresses of building as well as compressive stresses upto some extent of any building it also used to construct more safely earthquake resistant structurefor this construction we design various members of building such as beam, column, slab, footing,staircase,by considering factor of safety in which economy is also taken in account


Key Words- cement concrete,reinforced cement concrete,tensile\& compressive stresses,earthquake

resisting structure, factor of safety ,economy

## planofroval itiry

## 1.INTRODUCTION

There are many cities constructed in India and Maharashtra like amanora city in pune and magarpatta city in pune also a nanded city. The main purpose for construction of that cites is to improve the poshness of the area also to give entertain able things people and to give optional things for the people who have to take luxurious look by investing their money And hence by inspiring by that cities and to improve appearance of Shrirampur city area a work of surveying,estimating, and rcc designing for that city. This includes 3bhk apartments, 3bhk row houses, school. Collage, hostel, 1bhk apartment, in the 50 acres, and which named as royal city

### 1.1 Methods

## Steps use to find slab design

Step 1 if the shorter span lx
( < ) less then 3.5 m \& live load less then $3 \mathrm{kN} / \mathrm{m}^{2}$
Then , $D_{1}=1 x / 35$ ..for Fe 250
$\mathrm{D}_{\mathrm{l}}=1 \mathrm{x} / 0.8 \mathrm{X} 35$
.........forFe415
If the shorter $\operatorname{span} \mathrm{l}_{\mathrm{x}}(>)$ greater than 3.5 m
Live load $>3 \mathrm{kN} / \mathrm{m}^{2}$

$$
\begin{aligned}
& \mathrm{d}=\mathrm{lx} / 20 \mathrm{X} \mathrm{M}_{\mathrm{f}} \\
& \mathrm{D}=\mathrm{d}+\text { cover }
\end{aligned}
$$

Step 2-calculate effective span

$$
\begin{gathered}
\mathrm{l}_{\mathrm{x}}=\mathrm{l} \mathrm{x}+\mathrm{d} \\
\text { OR }
\end{gathered}
$$

c/c distance between two support whichever is less
step 3 - loading calculate
calculate factored laod (Wd) as calculated in simply supported slab
step 4 - calculate factored moment

$$
\begin{aligned}
& \mathrm{M}_{\mathrm{xd}}=\alpha_{\mathrm{x}} \mathrm{XW}_{\mathrm{d}} \mathrm{Xl}_{\mathrm{xe}}{ }^{2} \\
& \mathrm{M}_{\mathrm{yd}}=\alpha_{\mathrm{y}} \mathrm{XW}_{\mathrm{d}} \mathrm{Xl}_{\mathrm{xe}}{ }^{2}
\end{aligned}
$$

Where, $\mathrm{M}_{\mathrm{xd}}=$ factored moment about x -x axis
$\mathrm{M}_{\mathrm{yd}}=$ factoed moment about $y_{-} \mathrm{y}$ axis
$\alpha_{x} \& \alpha_{y}=$ bending moment coefficient for slab ( table no .
7.1 page no. 91 ) from IS 456-2000
step 5-determine required depth
d for bending
$\mathrm{M}_{\mathrm{d}}=0.149 \mathrm{FCK}_{\mathrm{bd}}{ }^{2}$ $\qquad$ for Fe 250
$\mathrm{M}_{\mathrm{d}}=0.138 \mathrm{FCK} \mathrm{bd}^{2}$ $\qquad$ for Fe 415
$\mathrm{M}_{\mathrm{d}}=0.133 \mathrm{FCK} \mathrm{bd}^{2}$ $\qquad$ .for Fe500
check $\mathrm{d}_{\text {(required) }}<\mathrm{d}_{\text {(Assume) }}$
step 6 - determine main steel -
A) Area of steel in $x$ - direction
$\mathrm{M}_{\mathrm{xd}}=0.87 \mathrm{fy} \mathrm{A}_{\mathrm{st}}\left(\mathrm{d}-0.42 \mathrm{X}_{\mathrm{u}}\right)$
Where, $\mathrm{xu}=\left(0.87 \mathrm{fy}_{\mathrm{st}}\right) /(0.36$ Fck b)
Find spacing $s_{x}=1000 \mathrm{XA} \mathrm{\phi} / \mathrm{A}_{\text {stx }}$
Where, $\mathrm{A} \phi=$ area of one bar
B) Area of steel in $Y$-direction
$\mathrm{M}_{\mathrm{yd}}=0.87$ fy $\mathrm{A}_{\text {sty }}\left(\mathrm{d}_{\mathrm{y}}-0.42 \mathrm{X}_{\mathrm{u}}\right)$
Where $d_{y}=d-\phi$
Find spacing $s_{y}=1000 \mathrm{XA} \mathrm{\phi} / \mathrm{A}_{\text {sty }}$
check for spacing $S_{x} \& S_{y} \nsucceq 3 d$ or 300 mm
step 7 - check for shear
as shear for in slab is very small no need to check for shear
step 8 - check for deflection
step 9 - summary

1) depth of slab
2) cover
3) main steel along $X$ - direction
4) main steel along $Y$ - direction
step 10 - reinforcement details


Fig. reinforcement detail of slab

## 2. Sample Problem

Step 1
if the shorter span $\mathrm{lx}(<)$ less then 3.5 m \& live load less then $3 \mathrm{kN} / \mathrm{m}^{2}$

Then ,
If the shorter span $l_{x}(>)$ greater than 3.5 m
Live load $>3 \mathrm{kN} / \mathrm{m}^{2}$

$$
\begin{aligned}
\mathrm{d} & =3500 / 20 \times 1.4 \\
& =125 \mathrm{~mm} \\
\mathrm{D} & =125+20 \\
& =145 \mathrm{~mm}
\end{aligned}
$$

Step 2- calculate effective span

$$
l_{\mathrm{xe}}=4+0.2=4.2
$$

OR
c/c distance between two support whichever is less step 3 - loading calculate
self wet $=1 \times \mathrm{D}(\mathrm{m}) \times 25$ $\qquad$
$=1 \mathrm{X} 0.145 \mathrm{X} 25$
$=3.625 \mathrm{KN} / \mathrm{m}$
Live load $=0.75 \mathrm{KN} / \mathrm{m}$
Floor finish $=0.75 \mathrm{KN} / \mathrm{m}$
........... 3

$$
\mathrm{w}=5.125 \mathrm{KN} / \mathrm{m}
$$

..... $1+2+3$
factored load $=1.5 \mathrm{X}$ w

$$
\text { = } 1.5 \times 5.125
$$

$$
=7.68 \mathrm{KN} / \mathrm{m}
$$

Aspect ratio $=l_{y} / \mathrm{l}_{\mathrm{x}}=4 / 3.5=1.14 \mathrm{~m}$

| Ly/lx | 1.1 | 1.2 |
| :---: | :---: | :---: |
| $\alpha_{x}$ | 0.074 | 0.084 |
| $\alpha_{y}$ | 0.061 | 0.059 |
| $\alpha_{x}=0.074+(0.084-0.074) /(1.2-1.1) \mathrm{X}(1.14-1.1)$ |  |  |
| $=0.078$ |  |  |

$\alpha_{y}=0.061+(0.059-0.061) /(1.2-1.1) X(1.14-1.1)$
$=0.0602$
step 4 - calculate factored moment

$$
\begin{aligned}
\mathrm{M}_{\mathrm{xd}} & =\alpha_{\mathrm{x}} \times \mathrm{W}_{\mathrm{d}} \times \mathrm{l}_{\mathrm{xe}}{ }^{2} \\
& =0.078 \times 7.68 \times 3.5^{2} \\
& =7.32 \mathrm{kN} / \mathrm{m} \\
\mathrm{M}_{\mathrm{yd}} & =\alpha_{\mathrm{y}} \times \mathrm{XW}_{\mathrm{d}} \times \mathrm{l}_{\mathrm{xe}}{ }^{2} \\
& =0.0602 \times 7.58 \times 3.5^{2} \\
& =5.58 \mathrm{kN} / \mathrm{m}
\end{aligned}
$$

Where, $\mathrm{M}_{\mathrm{xd}}=$ factored moment about x -x axis
$\mathrm{M}_{\mathrm{yd}}=$ factored moment about y_y axis
$\alpha_{x} \& \alpha_{y}=$ bending moment coefficient for slab ( table no .
7.1 page no. 91 ) from IS 456-2000
step 5-determine required depth
d for bending
$\mathrm{M}_{\mathrm{d}}=0.138 \mathrm{FCK}^{\mathrm{bd}}{ }^{2}$ $\qquad$ for Fe 415
$0.138 \times 20 \times 1000 \mathrm{~d}^{2}=7.62 \times 10^{6}$
$\mathrm{d}^{2}=7.62 \times 10^{6} / 2760$
$\mathrm{d}=52.54 \mathrm{~mm}<125 \mathrm{~mm}$
hence check ok
step 6 - determine main steel -
C) Area of steel in $x$-direction

$$
\begin{aligned}
& \mathrm{M}_{\mathrm{xd}}=0.87 \text { fy } \mathrm{A}_{\mathrm{st}}\left(\mathrm{~d}-0.42 \mathrm{X}_{\mathrm{u}}\right) \\
& 7.62 \mathrm{X} 10^{6}=0.87 \times 415 \mathrm{xA}_{\mathrm{st}}\left(125-0.42 \times 0.05 \mathrm{~A}_{\mathrm{st}}\right) \\
& = \\
& =361.05 \mathrm{Ast}(125-0.021 \mathrm{Ast}) \\
& =
\end{aligned} \begin{aligned}
& 75131.25 \mathrm{~A}_{\mathrm{st}}-7.58 \mathrm{~A}_{\mathrm{st}}{ }^{2} \\
& 7.58 \mathrm{~A}_{\mathrm{st}}{ }^{2}-45131.25+7.62 \times 10^{6}=0 \\
& \mathrm{~A}_{\mathrm{stx}}= 173.92 \mathrm{~mm}^{2}
\end{aligned}
$$

Where, $\mathrm{xu}=\left(0.87 \mathrm{fy} \mathrm{A}_{\mathrm{st}}\right) /(0.36$ Fck b)

$$
=\left(0.87 \mathrm{X} 415 \mathrm{X} \mathrm{~A}_{\mathrm{st}}\right) /
$$

$$
\begin{aligned}
& (0.36 \times 20 \times 1000) \\
= & 0.05 \mathrm{~A}_{\mathrm{st}}
\end{aligned}
$$

Find spacing $\mathrm{s}_{\mathrm{x}}=1000 \mathrm{XA} \mathrm{\phi} / \mathrm{A}_{\text {stx }}$

$$
\begin{aligned}
& =1000 \times 50.26 /(173.92) \\
& =288.98 \mathrm{~mm} \cong 280 \mathrm{~mm}
\end{aligned}
$$

Where, $\mathrm{A} \phi=$ area of one bar
D) Area of steel in $Y$-direction
$\mathrm{M}_{\mathrm{yd}}=0.87$ fy $\mathrm{A}_{\text {sty }}\left(\mathrm{d}_{\mathrm{y}}-0.42 \mathrm{X}_{\mathrm{u}}\right)$
$5.58 \times 10^{6}=0.87 \times 415 \times \mathrm{A}_{\text {sty }}\left(117-0.42 \times 0.05 \mathrm{~A}_{\text {st }}\right)$
$=361.05 \mathrm{~A}_{\text {sty }}\left(117-0.021 \mathrm{~A}_{\text {st }}\right)$
$=42242.85 \mathrm{~A}_{\text {sty }}-7.58 \mathrm{~A}_{\mathrm{st}}{ }^{2}$
$7.58 \mathrm{~A}_{\text {st }^{2}-42242.85 \mathrm{~A}_{\text {sty }}+5.58 \times 106=0}$
$\mathrm{A}_{\mathrm{sty}}=135.38 \mathrm{~mm}^{2}$
Where $d_{y}=d-\phi$

$$
\begin{aligned}
& =125-8 \\
& =117 \mathrm{~mm}
\end{aligned}
$$

Find spacing $s_{y}=1000 \times \mathrm{A} \phi / \mathrm{A}_{\text {sty }}$

$$
\begin{aligned}
& =1000 \times 50.26 / 135.38 \\
& =371.25 \mathrm{~mm} \cong 340 \mathrm{~mm}
\end{aligned}
$$

check for spacing $S_{x} \& S_{y} \nsucceq 3 d$ or 300 mm

$$
\begin{gathered}
\mathrm{S}_{\mathrm{x}} \nsucceq 3 \mathrm{~d} \text { or } 300 \\
280 \nsucceq 3 \times 125 \\
280 \succcurlyeq 375 \\
\mathrm{~S}_{\mathrm{y}}^{\mathrm{y}} \not \mathrm{Z} \text { dy or } 300 \\
340 \nsucceq 3 \times 117 \\
340 \succcurlyeq 351
\end{gathered}
$$

step 7 - check for shear
as shear for in slab is very small no need to check for shear
step 8 - check for deflection
step 9 - summary
5) depth of slab (D) $=145 \mathrm{~mm}$
6) cover $=20 \mathrm{~mm}$
7) main steel along $X$ - direction

8mmф@280mmc/c
8) main steel along Y - direction

8mmф@340mmc/c
step 10 - reinforcement details


Fig. reinforcement detail of slab

## 3. CONCLUSIONS

By use of Reinforced cement concrete (R.C.C)we construct the safe and earthquake resisting structure. By considering various aspects such as factor of safety and economy we design the city effectively

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## BIOGRAPHIES (Optional not mandatory )



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