# PLANNING AND ANALYSIS OF IRRIGATION TUNNEL 

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#### Abstract

The paper deals with the planning and analysis of airrigation tunnel. The tunnel is designed in D shape. D shape resembles like rectangular shape in the bottom portion called benching and the semicircular shape in the top portion called heading.


The tunnel is laid in rock surface is about 10 Kms . The width of the tunnel is 6.5 m and the height of the tunnel is 7.15 m . The sequence of the project that we followed are estimation of water quantity discharge, arrival of cross section, analysis of cross section, design of cross section, rate analysis of tunnel cross section. The drawing plan is done using AutoCAD software.

The analysis is carried out using Staad pro software. Staad pro analysis is mainly used to evaluate load. Design procedure is done as per IS codes.

Keywords:Irrigation tunnel, AutoCAD plan, staad pro analysis, rate analysis.
1.INTRODUCTION (Size 11 , cambria font)

Tunnel can be defined as underground passages made without removing the overlying rock or soil. It serves as many functions - Highway, Rail road or rapid transit artery; pedestrian passage way, fresh water conveyance, cooling water supply, waste water collector, underground storage or transport, hydro power generator, utility corridor etc. A tunnel can be located in any of a variety of places - under mountains, cities, rivers, lakes, sea estuaries, straits, or bays.

A tunnel is constructed in one of innumerable media-soft ground, mixed face, rock, and uniform, jumbled, layered, dry, wet, stable, flowing, and squeezing.

### 1.1. SCOPE AND OBJECTIVE

The ultimate objective is
$>$ To analyze and design the irrigation tunnel that provides water for drought areas.
$>$ To use AutoCAD and Staad pro software effectively to design and analyze the various components of the tunnel and also by manual design.
$>$ To learn the staad pro Software.
$>$ To learn the AutoCAD software.
$>$ To perform a job in a challenging environment for better achievement.

### 1.2. Drilling patterns

> Horizontal wedge cut
$>$ Pyramid cut
$>$ Fan cut
$>$ Vcut
Among the various above drilling
Patterns we adopted V cut drilling pattern for our analysis purpose.


### 1.3 Methodology

The following are the works to be done in this project.


## 2.PLAN

### 2.1 Plan of tunnel



Fig 2.1. Plan view of tunnel
2.2. Cross section


Fig 2.2 cross section

### 2.3.3D view of tunnel



Fig 3.3. 3D view of tunnel
3.ESTIMATION DETAILS

### 3.1.ESTIMATION OF WATER QUANTITY DISCHARGE

Area of excavation $=(6.5 \times 3.9)+\left(\Pi \times(2.35 / 2)^{2}\right)$

$$
=29.69 \mathrm{~m}^{2}
$$

Area of water flowing $=(4.7 \times 3)$

$$
=14.10 \mathrm{~m}^{2}
$$

Discharge of water, $\mathrm{Q}=\mathrm{AV}$
Assume, $\mathrm{V} \quad=1.5 \mathrm{~m} / \mathrm{s}$

$$
Q=14.10 \times 1.5
$$

$$
\mathrm{Q}=21.15 \mathrm{~m}^{3} / \mathrm{s}
$$

### 3.2.QUANTITY OF CONCRETING

Area of concreting = Area of excavation- Area of water flowing
= 29.69 - 14.10
$=15.59 \mathrm{~m}^{2}$
Quantity of concreting
for $10 \mathrm{~km}=15.59 \times 10000$

$$
=155900 \mathrm{~m}^{3}
$$

## 4. STAAD PRO ANALYSIS

Analysis is the process of determination of forces in the members of the structures due to external loads acting on it.

Load used

1. Dead load
2. Liveload
3. 1.5 ( dead load+ live load)

Plate thickness used

1. Top roof 350 mm
2. Side walls 250 mm

Type of concrete used is M40 grade Type of steel used is Fe 500 tmt bar

### 4.1.Post processing results

The result output in graphical form is represented as below

### 4.1.1. Dead load:

The selfweight of the reinforced wall element is taken as dead load of slab and in addition the top layer and side layer of rock also considered for the study


Fig 4.1.dead load

### 4.1.2. Max moment 1.5(DL+LL)

Factored combination load of $1.5(\mathrm{Dl}+\mathrm{LL})$ is considered.



Fig 4.2.max moment

### 4.1.3.Live load

Load due to any movement at top of the tunnel or additional pressure caused by water is used for the live load.


### 4.1.4.Displacement



Fig 4.4. Displacement

### 4.1.5.Loading details of the baseslab in tunnel.

The portion of the slab is divided in to multiple portion and support condition is given for analysis

| Node | L/C | Fy kN |
| :---: | :---: | :---: |
| 1 | 3 COMBINATION LOAD <br> CASE 3 | 82.348 |
| 2 | 3 COMBINATION LOAD <br> CASE 3 | 82.348 |
| 5 | 3 COMBINATION LOAD <br> CASE 3 | 82.346 |
| 38 | 3 COMBINATION LOAD <br> CASE 3 | 82.346 |
| 39 | 3 COMBINATION LOAD <br> CASE 3 | 142.517 |
| 40 | 3 COMBINATION LOAD <br> CASE 3 | 144.553 |
| 41 | 3 COMBINATION LOAD <br> CASE 3 | 144.459 |
| 42 | 3 COMBINATION LOAD <br> CASE 3 | 144.459 |
| 43 | 3 COMBINATION LOAD <br> CASE 3 | 144.467 |


| 44 | 3 COMBINATION LOAD CASE 3 | 144.553 |
| :---: | :---: | :---: |
| 45 | 3 COMBINATION LOAD CASE 3 | 142.519 |
| 46 | $3 \text { COMBINATION LOAD }$ CASE 3 | 142.517 |
| 47 | $3 \text { COMBINATION LOAD }$ <br> CASE 3 | 144.553 |
| 48 | 3 COMBINATION LOAD CASE 3 | 144.467 |
| 49 | $3 \text { COMBINATION LOAD }$ CASE 3 | 144.459 |
| 50 | $3 \text { COMBINATION LOAD }$ CASE 3 | 144.453 |
| 51 | $3 \text { COMBINATION LOAD }$ <br> CASE 3 | 144.459 |
| 52 | $3 \text { COMBINATION LOAD }$ CASE 3 | 144.467 |
| 53 | $3 \text { COMBINATION LOAD }$ CASE 3 | 144.553 |
| 54 | 3 COMBINATION LOAD CASE 3 | 142.519 |
| 55 | 3 COMBINATION LOAD CASE 3 | 35.911 |
| 56 | 3 COMBINATION LOAD CASE 3 | 35.911 |
| 84 | $3 \text { COMBINATION LOAD }$ CASE 3 | 71.822 |
| 85 | $3 \text { COMBINATION LOAD }$ CASE 3 | 71.822 |
| 86 | $3 \text { COMBINATION LOAD }$ CASE 3 | 71.822 |
| 87 | $3 \text { COMBINATION LOAD }$ CASE 3 | 71.822 |
| 88 | $3 \text { COMBINATION LOAD }$ CASE 3 | 71.822 |
| 89 | $3 \text { COMBINATION LOAD }$ CASE 3 | 71.822 |
| 90 | $3 \text { COMBINATION LOAD }$ CASE 3 | 71.822 |
| 91 | $3 \text { COMBINATION LOAD }$ <br> CASE 3 | 71.822 |
| 92 | $3 \text { COMBINATION LOAD }$ <br> CASE 3 | 71.822 |

Maximum shear force $=-20.268 \mathrm{kN}$
Minimum bending moment $=-20.231 \mathrm{kNm}$
144.459

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## 5. RATE ANALYSIS OF TUNNEL

### 5.1. BASIC DETAILS

| 1 | Cross Sectional Area $=$ | $14.1 \mathrm{~m}^{2}$ |
| :--- | :--- | :--- |
| 2 | Average Pull $/$ Cycle $=$ | 1.5 m |
| 3 | Quantity / Blast $=$ | 14.1 x 1.5 |
| 4 | No of Holes Drilled $=$ | 16 Holes |
| 5 | Avg Depth of Holes $=$ | $7^{\prime}=2.1 \mathrm{~m}$ |
| 6 | Drilling length/Cycle $=$ | 2.1 mx 16 |

### 5.2.Total Cycle Hours / Blast

| No of Jack Hammers Used $=$ | 4 Nos <br> 3 hrs |
| :--- | :--- |
| Drilling Time $=$ | 45 min |
| Blowing Time $=$ | 7 min |
| Blasting material Loading \& |  |
| Blasting= | 45 min |
| Defuming= | 44 min |
| Primary Scaling= | 16 min |
|  | 3 hrs |
| Mucking= | 49 min |
|  | 1 hr |
| Secondary Scaling= | 02 min |
| Survey( Profile Marking)= | 31 min |
| Pipeline \& Drilling |  |
| Arrangement= | 25 min |
|  | $\mathbf{1 1 \mathrm { hrs }}$ |
| Total Hours / Cycle | $\mathbf{1 5 m i n}$ |

### 5.3.RATE DETAILS

| S. <br> No | DESCRIPTION | TOTAL <br> RATE IN <br> Rs. | RATE <br> PER CUM <br> IN Rs. |
| ---: | :---: | :---: | :---: |
| 1 | power(Drilling and <br> Blasting) | 10990 | 519.62 |
| 2 | Explosives per <br> blast | 17560.7 | 830.29 |
| 3 | Drill rods | 731.80 | 34.6 |
| 4 | Excavator\{Tata <br> hitachi\} | 1321.40 | 62.47 |
| 5 | Compressor | 2826.61 | 133.65 |
| 6 | Tippers | 499.432 | 23.61 |
| 7 | Generators | 3691.18 |  |
| 8 | Jumbo | 38.88 | 174.52 |
|  |  | 1.84 |  |


| MACHINERIES |  |  |  |
| :---: | :---: | :---: | :---: |
| 9 | Excavator\{Tata <br> hitachi\} | 2232.42 | 446.48 |
| 10 | Compressor | 456.75 | 91.35 |
| 11 | Tippers | 4063.50 | 135.45 |
| 12 | Generators | 579.08 | 57.92 |
| 13 | Jumbo $14.10 \mathrm{~m}^{2}$ | 894.16 | 149.03 |
| 14 | Jack ham5ms | 150.10 | 5 |
| 15 | Ventilation | 1127.41 | 225.48 |
| 16 | Pipe lining | 246.6 | 82.2 |
| 17 | Dewatering | - | 3 |
| 18 | Dewatering <br> pipeline | 236.56 | 63.64 |
| 19 | Miscellaneous | 154.04 | 51.35 |

Total rate analysis per cum is Rs.3993.69/-

## 6.CONCLUSION

The conclusion of this paper are given as,
$>$ All the drawings in this project were drafted using AutoCAD 2007 software.
$>$ The analysis of irrigation tunnel is done by using Staad pro software.
$>$ Using the staad pro software we learnt the load calculations and withstanding capability of the tunnel.
$>$ The complete rate analysis of men, materials and machineries for this project was done according to the schedule of rates.
$>$ And also the estimation of water quantity discharge and the quantity of concreting was done.
> From this project, the complete procedure for constructing the tunnel analytically was learnt. This has created a good awareness about the entire process of tunnel construction.

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## 7. REFERENCES

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[2] IS 875-(Part-1):1987, Indian Standard code of practice for design loads (Dead Load), second reversion, New Delhi.
[3] IS 875-(Part-2):1987, Indian Standard code of practice for design loads (Live Load), second reversion, New Delhi.
[4] IS 5878 Part II /Section-2 for Dewatering of tunnel.

