Strength and Durability Studies on Concrete made using Treated Recycled Coarse and Fine Aggregate

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Abstract: The fast development of human civilization has resulted in fast depletion of natural resources and accretion of manmade waste. Whereas in construction Industry for the fast decades it witnessed a fast depletion of Natural available materials like Coarse aggregate and fine aggregate, whereas in regards the fine aggregate i.e. sand, which has become a scarce resource and key to economic development in the 21st century. As a measure of conversation of these valuable resources, studies have been conducted on recycled demolished concrete aggregates for its complete usage in concrete. The method adopted for its usage in concrete is mainly consists of improving the structure, mechanical and physical properties of aggregate. The structure has been improved by crushing the aggregate in Impact Jaw crusher and also through the ball mill method were the surface defects of coarse aggregate were removed and also the cement hydrated product adhered on the aggregates were densified and strengthened by using various Chemicals such as Sodium Silicate, Lithium Silicate, Colloidal Silica dioxide, Silane Siloxane. Stearic Acid and Kaolin + PVOH. In each chemical treatment we have imparted certain modification of aggregate characteristics and finally a combination has been arrived were the aggregated combination i.e. coarse aggregate treated with lithium silicate and fine aggregate treated with colloidal silica dioxide has given at par result with Natural aggregates both from strength and durability point of view specialise in its usage in Cement mortar and in Concrete.

Key Words: Strength, Durability, Recycled aggregates, Treated aggregates, Concrete.

1. INTRODUCTION

1.1 General

Since, the usage of these natural stones and sand is at the tune of 90% in concrete production, which intern being the third maximum consuming material on earth after water and air. Thus, an alternative material usage in places of these aggregates will be boon to the environment and to life of human civilization. The introduction of usage of Industrial waste like construction demolished old concrete

in place of coarse and fine aggregate in concrete could resulted in a sustaining reduction of our dependency of natural material. Many researches have been done in these areas; which resulted in limited contribution, as their recommendation portions to limited usage of these material in concrete industry.

Our research emphases on cent percentage utilization of recycled fine and coarse aggregate in construction industry. Which involves an improvement technique and surface treatment on improving the physical and mechanical properties of recycled demolished concrete aggregates. Since, the main scope of research work is on exploring the improvement techniques & surface modification methods which can results in 100% utilization of demolished recycled fine and coarse aggregate as an alternative material to natural aggregate in its use in construction industry.

From literature review, it has been inferred that these hydrated cement particles on aggregate surface is of porous nature and has high water absorption properties and also these hydrate cement surface on aggregate is a combination of calcium silicate hydrate(C-S-H) and Calcium Hydrate (Ca(0H)2) Chemical. To control porosity and to arrest water absorption properties, we should control the formation of Ca(OH)2 and for controlling these, we should know the chemistry of Cement Hydration. As cement hydrates, it produces calcium silicate hydrate(C-S-H).Basically, as concrete hardens, water reacts with cement to form calcium hydroxide and silica, which reacts to form C-S-H, and these material bonds the cement with the aggregate in concrete .But the hydration process produces more calcium hydroxide than is used up in this chemical reaction, resulting in excess calcium hydroxide also called free lime. Over the long term, excess free lime is troublesome because it is primary causes of micron pitting. These excess calcium hydroxides along with hardened C-S-H constitute main parts of hydrate cement particles. These excess lime from hydrated cement, will be used in closing the pores and also in densifying the recycled aggregate particles. Our research emphasis is on how good theses excess lime present in concrete demolished recycled aggregate is put to use.

1.2 Objectives of the work

- To evaluate the reasons for weak zone in recycled aggregate and the reasons for its restricted usage in concrete industry.
- To arrive at a best surface treatment methodology were the internal and external surface of the recycled aggregate can be improved.
- on recycled coarse and fine aggregate that can be used as percentage replacement of Natural aggregates in Cement Mortar and in Concrete without compromising on strength and durability aspects.

1.3 Scope of the work

- To find a treatment methodology for coarse and fine aggregate where there is significant improvement in properties of aggregate.
- To study the strength parameters on Recycled Fine aggregate as a complete replacement material for Sand in Cement Mortar.
- To find the strength related parameters on hardened concrete made of treated aggregates which mainly comprises of Compressive strength, Split Tensile Strength and Flexural strength.
- To study the durability aspects on hardened concrete which mainly consists of assessing the strength of the concrete which are exposed to sulphate, acid and chloride attack.

1.4 Literature Review

The present investigation work is identified as the investigation of properties of concrete made using variable materials and its possible enhancement. To evaluate the reasons for weak zone in recycled aggregate and the reasons for its restricted usage in concrete industry. To arrive at a best surface treatment methodology were the internal and external surface of the recycled aggregate can be improved. To obtain a combination of treatment methodology on recycled coarse and fine aggregate that can be used as percentage replacement of Natural aggregates in Cement Mortar and in Concrete without compromising on strength and durability aspects and some main investigations are observed.

T. Parluy and M. Sefflova: conducted studies on strength development of fine aggregate concrete with recycled cement powder, the study mainly deals with possibility of use of cement powder of recycled concrete aggregates as partial replacement of cement in concrete mixture. The main topic of this article is the study of evaluation of the

mechanical properties of the fine aggregates concrete with partial cement replacement by recycled concrete powder in time. The compressive strength, tensile strength in bending and dynamic modulus of elasticity were tested at the age 7, 14 & 28 days. The fine recycled concrete which was used as partial replacement of cement had the same grain size as cement. The replacement rate was 5%, 10% and 15%. Mechanical properties were investigated by using cubic and prismatic specimens.

Amnon Katz: Amnon Katz conducted experimental studies on treatment methods to be adopted for the improvement of recycled aggregate, the microstructure of recycled aggregate prepared from the crushing of old concrete was studied. It was found that the recycled aggregate is covered with lose particles that may prevent good bonding between the new cement matrix and the recycled aggregate. The old cement paste that remained on the natural aggregate was porous and cracked, leading to weak mechanical properties of the recycled aggregate. Treatment of the recycled aggregate by impregnation of silica fume solution and by ultrasonic cleaning was studied with the objective of overcoming the above-mentioned limitations. An increase of ~30 and ~15% in the compressive strength at ages 7 and 28 days was observed after the silica fume treatment.

2. MATERIALS and METHODOLOGY

2.1 Material Used

In the present study material used are Water, Cement, Super plasticizer, Sand, M-Sand, Recycled Fine aggregate, Recycled Coarse aggregate.

2.1.1 Cement

Cement used is ordinary Portland Cement of 53 Grade. The specific gravity of cement is 3.14 The test conducted on cement for different properties are presented in Table 2.1. The Cement used is as per IS: 256.

SI. No.	Test	Results	Requirements as per IS-12269:2013	
1	Fineness of	3450	3200 cm ² /g	
T	cement	cm2/g	5200 cm /g	
2	Normal	29%	Not Specified	
2	Consistency	29%	Not Specified	
3	Soundness (Le-	05 mm	Shall not be more	
3	chatelier's)	0.5 mm	than 10 mm	
4	Specific gravity	3.14	Not Specified	
	Setting time (in minutes)			
	Initial Setting	105	Shall not be less than	
5	time	minutes	30 minutes	
	Final Setting	375	Shall not be more	

Table 2.1: Physical Properties of 53 grade Cement



	Time	minutes	than 600 minutes	
	Compressive Strength (MPa) (70.5 x 70.5 x 70.5 mm cubes)			
	,	38.7	Shall not be less than	
	3 Days Strength.	МРа	27.0 MPa	
6	7 Days Strength	48.2	Shall not be less than	
0		МРа	37.0 MPa	
	20 Davia Chron ath	56.8	Shall not be less than	
	28 Days Strength	МРа	53.0 MPa	

2.1.2 Superplasticizer

Readily available Superplasticizer of FOSROC make Conplast 450 has been used for the present work.

2.1.3 Fine Aggregate

2.1.3.1 Natural Fine Aggregate (NFA)

In the present study, the river sand from Chitoor confirming to Zone-II gradation has been Used. The physical Properties of the river sand are tabulated at Table 2.2. The sand used is as per IS :383

2.1.3.2 M-sand

M-sand confirming to Zone –II has been used which has been procured from nearly M-Sand Suppliers, M-sand used is as per IS:383 and are tabulated at Table 2.2

2.1.3.3 Recycled Fine Aggregate (RFA)

Recycled Fine aggregates were obtained from C&D waste Processing Unit in Bangalore Outskirts at Chik Jala i.e. from Rock Crystals. In these Unit the construction and other demolished waste are crushed through Impact jaw crushers and through cylindrical sieves the aggregates are processed into Coarse and fine aggregate of Different sizes. The recycled fine aggregates obtained from the above unit was confirming to Zone –II. The physical properties of the recycled fine aggregate are tabulated at Table 2.2

Table 2.1: Physical Characteristics of Fine Aggregates

Physical Properties	NFA	M-sand	RFA
Size	4.75mm	4.75mm	4.75mm
Specific Gravity	2.65	2.60	2.13
Fineness Modulus	3.70	2.73	1.69
Bulk density	1680 kg/m ³	1445 kg/m ³	1340 kg/m ³
Water absorption	1.01%	1.21%	6.38%

2.1.4 Coarse Aggregate

2.1.4.1 Natural Coarse Aggregate (NCA)

The granite metal jelly 20 and down size were obtained from stone crusher near Bangalore. Coarse aggregate crushed granite of 20mm maximum size and retained on IS480 sieves is been used. The sieve analysis of coarse aggregate conforms to the Specifications of IS 383:1970 for graded aggregates and specific gravity. The test results of tests are tabulated in Table 2.3

2.1.4.2 Recycled Coarse Aggregate (RCA)

Recycled coarse aggregates were obtained from C&D waste Processing Unit in Bangalore Outskirts at Chik Jala i.e. from Rock Crystals. In these Unit the construction and other demolished waste are crushed through Impact jaw crushers and through cylindrical sieves the aggregates are processed into Coarse and fine aggregate of Different sizes. The recycled coarse aggregates obtained from the above unit was confirming to Zone –II. The physical properties of the recycled coarse aggregate are tabulated at Table 2.3

Table 2.3: Physical Properties of Coarse Aggregate

Properties of Tested	Test Results (NCA)	Test Results (RCA)	
Specific Gravity	2.62	2.29	
Bulk Density	1595 kg/m ³	1446 kg/m ³	
Fineness Modulus	4.72	5.90	
Water Absorption	0.5%	4.5%	
Crushing Value of Aggregate	23.82	26.36	
Aggregate Impact Value	22.41	27.16	
Abrasion value	21.15	29.63	

2.1.5 Water

Clean potable water is used for mixing and curing of concrete.

2.1.6 Chemicals Used for the Surface Treatment of Aggregate

- Sodium Silicate
- Lithium Silicate
- Colloidal Silicon dioxide
- Silane Siloxane
- Polyvinyl alcohol (PVOH or PVA)
- Kaolin
- Stearic acid

3. EXPERIMENTAL INVESTIGATION

3.1 Compressive Strength

The compressive strength of concrete i.e. ultimate strength of concrete is defined as the load which causes failure of the specimen divided by the area of the cross section in uniaxial compression, under a given rate of loading. (Fig 3.1)

The compression strength is calculated using the formula,

$f = \frac{P}{4} N/mm^2$

Where, f = is compressive strength of concrete in N/mm²

P = ultimate load resisted by concrete in N

A = area of specimen in mm^2

3.2 Flexural Strength

When concrete is subjected to bending, Tensile, compressive stresses and in many cases direct shearing stresses occur. The most common example of concrete structure subjected to flexure are highway pavements and the strength of concrete for pavements is commonly evaluated by means of bending tests on 100x100x500mm beam specimens. Flexural strength is expressed in terms of "Modulus rupture" which is the maximum tensile (or compressive) stress at rupture. (Fig 3.2)

$$\mathbf{f}_{\mathbf{b}} = \frac{(\mathbf{p} \times \mathbf{l})}{b \times d^2}$$

Where,

f_b= modulus of rupture, N/mm²

b = measured width in mm of the specimen

d = measured depth in mm of the specimen at the point of failure

l = length in mm of the span on which specimen was supported

p = max. Load in N applied to the specimen

3.3 Split Tensile Strength

The tensile strength of concrete is one of the basic and important properties which greatly affect the extent and size of cracking in structures. Moreover, the concrete is very weak in tension due to its brittle nature. Hence, it is not expected to resist the direct tension. Therefore, it is necessary to determine the tensile strength of concrete to determine the load at which the concrete members may crack. (Fig 3.3)

The cylinders were tested in compression testing machine in 2000kN capacity. The test specimen was loaded at constant rate of loading at 1.2N/cm²/min to 2.40N/cm²/min as per the standard procedure explained in IS 5816:1999

 $T = \frac{2P}{\pi DL}$

Where.

T = splitting tensile strength in MPa

P = max. applied load indicated by the testing machine in N

D = diameter of specimen in mm

L = length of the specimen in mm

4. RESULTS AND DISCUSSION

4.1 General

From the test results it can be inferred that each treatment technique has its own impact on the surface modification and in improving the properties of the aggregates. It was studied on recycled coarse and fine aggregate by treating the aggregates with above mentioned chemicals solution in certain dilution ratios shown in below tables i.e. the aggregates were soaked for 24 hours in the diluted chemicals and after 7 days of curing. where tested for physical properties of the aggregates i.e. mainly Water absorption, Specific Gravity and Fineness modulus. From the test results were tabulated in following tables and the same procedure is followed for all chemicals.

4.2 Sodium Silicate Treatment

It can be observed that increasing the dosing content of lithium silicate has increased the specific gravity and density up to 5% of dosage of lithium silicate, beyond the above the values are reduced. But for water absorption the value got decreased up to 5% of dosing (table 4.1)

Table 4.1: Properties of RCA with Sodium Silicate (Na_2Sio_3)

Properties of RCA+Na ₂ Sio ₃ Tested	2.5% of Na ₂ Sio ₃	5% of Na ₂ Sio ₃	7.5% of Na ₂ Sio ₃	10% of Na ₂ Sio ₃
Specific Gravity	2.59	3.00	3.05	3.28
Bulk Density	1320	1335	1330	1335
Fineness Modulus	4.72	4.72	4.72	4.72
Water absorption	2.43%	1.87%	2.42%	2.06%

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e-ISSN: 2395-0056 p-ISSN: 2395-0072

In case of fine aggregates, the treatment of lithium silicate has quite opposite effects as it was shown on coarse aggregate. From the results as tabulated at table 4.2 it can be inferred that Same proportion of dilution ratio has resulted increase in Specific gravity with increase in dosing content whereas the water absorption decreased with increase in dosing which was quite reverse to that seen with treatment with sodium silicate.

Table 4.2: Properties of RFA with Sodium Silicate
(Na ₂ Sio ₃)

Properties of RFA+Na ₂ Sio ₃ Tested	2.5% of Na2Sio3	5% of Na2Sio3	7.5% of Na2Sio3	10% of Na2Sio3
Specific Gravity	2.51	2.40	2.39	2.30
Bulk Density	1450	1390	1380	1380
Fineness Modulus	3.70	3.70	3.70	3.70
Water absorption	3.6%	6.6%	7.7%	9.64%



Fig 3.1: Compressive testing

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Fig 3.3: Split tensile testing

machine

machine

Fig 3.2: Flexure strength testing

machine

4.3 Lithium Silicate Treatment

It can be observed that increasing the dosing content of lithium silicate has increased the specific gravity and density up to 5% of dosage of lithium silicate, beyond the above the values are reduced. But for water absorption the value got decreased up to 5% of dosing. The optimum dosage where there is increase in Specific Gravity and decrease in Water absorption when compared with that of Natural coarse aggregate is 5% of Lithium silicate treatment on Coarse Aggregate. (table 4.3)

Table 4.3: Properties of RCA with Lithium Silicate
(Li ₂ Sio ₃)

Properties of RCA+Li ₂ Sio ₃ Tested	2.5% of Li ₂ Sio ₃	5% of Li2Sio3	7.5% of Li ₂ Sio ₃	10% of Li2Si03
Specific Gravity	2.86	2.90	2.81	2.84
Bulk Density	1420	1440	1480	1465
Fineness Modulus	4.72	4.72	4.72	4.72
Water absorption	1.74%	1.41%	3.34%	3.30%

In case of fine aggregates, the treatment of lithium silicate has quite opposite effects as it was shown on coarse aggregate. From the results as tabulated at table 4.4 it can be inferred that Same proportion of dilution ratio has resulted increase in Specific gravity with increase in dosing content whereas the water absorption decreased with increase in dosing which was quite reverse to that seen with treatment with lithium silicate.

Table 4.4: Properties of RFA with Lithium Silicate(Li2Sio3)

Properties of RFA+Li ₂ Sio ₃ Tested	2.5% of Li ₂ Sio ₃	5% of Li2Sio3	7.5% of Li2Sio3	10% of Li2Sio3
Specific Gravity	2.46	2.43	2.43	2.49
Bulk Density	1430	1400	1410	1450
Fineness Modulus	3.70	3.70	3.70	3.70
Water absorption	3.51%	3.09%	3.73%	2.66%

4.4 Colloidal Silica Dioxide Treatment

It can be observed that increasing the dosing content of silica dioxide it has increased the specific gravity and density up to 5% of dosage of Silica Dioxide, beyond the above the value reduced. But for water absorption the value got decreased up to 2.5% of dosing and thereafter the water absorption value got increased with increase in dosing content. (table 4.5)

Table 4.5: Properties of RCA with Colloidal SilicaDioxide (Sio2)

Properties of RCA+Sio ₂ Tested	1% of Sio ₂	2.5% of Sio ₂	5% of Sio2	7.5% of Sio ₂
Specific Gravity	2.82	2.81	2.68	2.71
Bulk Density	1450	1410	1380	1360
Fineness Modulus	4.72	4.72	4.72.	4.72
Water absorption	2.02%	2.43%	3.34%	3.16%

In case of fine aggregates, the treatment of Colloidal silica dioxide has quite opposite effect as it was shown on coarse aggregate. From the results as tabulated at table 4.6 it can be inferred that same proportion of ratio has resulted in decrease in Specific gravity with increase in dosing content and, whereas the water absorption increased with increase in dosing.

Table 4.6: Properties of RFA with Colloidal Silica
Dioxide (Sio ₂)

Properties of RFA+Sio ₂ Tested	1% of Sio ₂	2.5% of Sio ₂	5% of Sio2	7.5% of Sio ₂
Specific Gravity	2.53	2.56	2.57	2.35
Bulk Density	1400	1430	1450	1380
Fineness Modulus	3.70	3.70	3.70	3.70
Water absorption	1.62%	1.62%	2.04%	2.66%

4.5 Silane Siloxane Treatment

From the test results as tabulated at table 4.7 it can be observed that increasing the dosing content of Silane Siloxane it has decreased the specific gravity and density. But for water absorption the values are varied with the dosing content and are compared with the natural aggregate values.

Table 4.7: Properties of RCA with Silane Siloxane

Properties of RCA+SS Tested	10% of SS	20% of SS	30% of SS	40% of SS
Specific Gravity	2.51	2.57	2.67	2.54
Bulk Density	1295	1315	1330	1320
Fineness Modulus	4.72	4.72	4.72	4.72
Water absorption	0.49%	0.39%	0.90%	0

In case of fine aggregates, the treatment of stearic acid has similar effects as shown on coarse aggregate. From the results are tabulated at table 4.8 it can be inferred that same proportion of dilution ratio has resulted in decrease in Specific gravity with increase in dosing content and, whereas the water absorption varying with dosing.

Table 4.8: Properties of RFA with Silane Siloxane

Properties of RFA+SS Tested	10% of SS	20% of SS	30% of SS	40% of SS
Specific Gravity	2.20	1.81	1.78	1.75
Bulk Density	1280	1265	1255	1240
Fineness Modulus	3.70	3.70	3.70	3.70
Water absorption	0.80%	4.60%	0.60%	0.80%

4.6 Polyvinyl Alcohol with Kaolin Treatment

From the test results as tabulated at table 4.9 it can be observed that increasing the dosing content there is decrease in the specific gravity and density. But for water absorption the value got increased with increase in dosing content. The optimum dosage 1% and 1.5% and are compared with natural coarse aggregate.

Table 4.9: Properties of RCA with Polyvinyl alcohol +Kaolin (PVOH+KAOLIN)

Properties of RCA+PVOH+KAO LIN Tested	10% PVOH+ 1%KAOL IN	10% PVOH+ 1.5%KAOL IN	10% PVOH+ 2%KAOL IN
Specific Gravity	2.34	2.52	2.53
Bulk Density	1295	1315	1340
Fineness Modulus	4.72	4.72	4.72
Water absorption	1.62	1.72	2.33

In case of fine aggregates, the treatment of Kaolin has similar effects as it was shown on coarse aggregate. From the results as tabulated at table 4.10 it can be inferred that same proportion of dilution ratio has resulted in decrease in Specific gravity with increase in dosing content and the water absorption is constant.

Table 4.10: Properties of RFA with Polyvinyl alcohol +
Kaolin (PVOH+KAOLIN)

Properties of RFA+PVOH+KAO LIN Tested	10% PVOH+ 1%KAOL IN	10% PVOH+ 1.5%KAOL IN	10% PVOH+ 2%KAOL IN
Specific Gravity	2.42	2.23	2.27
Bulk Density	1340	1325	1365
Fineness Modulus	3.70	3.70	3.70
Water absorption	1.21%	1.21%	1.21%

4.7 Stearic Acid Treatment (SA)

From the test results as tabulated at table 4.11 it can be observed that increasing the dosing content of Steric acid it has decreased the specific gravity and density. But for water absorption the value got increased with increasing in dosing content. The optimum dosage where there is increase in Specific Gravity is 1% and decrease in Water absorption is 1% when compared with that of Natural coarse aggregate.

Table 4.11: Properties of RCA with Stearic Acid (CH3(CH2)16COO-)

Properties of RCA+SA	1% SA	2.5% SA	5% SA	7.5% SA
Specific Gravity	2.61	2.56	2.53	2.50
Bulk Density	1295	1275	1280	1250
Fineness Modulus	4.72	4.72	4.72	4.72
Water absorption	1.71%	2.0%	2.33%	2.26%

In case of fine aggregates, the treatment of stearic acid has similar effects as it was shown on coarse aggregate. From the results as tabulated at table 4.12 it can be inferred that same proportion of dilution ratio has resulted in decrease in Specific gravity with increase in dosing content and the water absorption increased with increase in dosing.

Table 4.12: Properties of RFA with Stearic Acid (CH3(CH2)16COO-)

Properties of RFA+SA	1% SA	2.5% SA	5% SA	7.5% SA
Specific Gravity	2.43	2.07	2.04	2.06
Bulk Density	1380	1320	1310	1315
Fineness Modulus	3.70	3.70	3.70	3.70
Water absorption	2.24%	3.51%	4.82%	4.60%

Based on the above test results it can be concluded that lithium silicate and colloidal silica dioxide and Silane siloxane treatment is effective on Coarse aggregate and Colloidal Silica dioxide treatment for fine aggregate. Therefore, its suitability in concrete application combination of treatment has been arrived and same has been compared with the concrete prepared with Natural aggregates and with recycled aggregates.

Table 4.13: Compressive Strength Mortar cube treatedwith different chemicals

SI. No	Cube type	Weight (grams)	Compressive Strength (MPa)
1	NATURAL (N)	836	30.12
2	M-SAND (M)	821	24.42
`3	RECYCLED (R)	815	21.08
	RFA+ Na ₂ Sio ₃		
4	2.5% of Na ₂ Sio ₃	828	25.24
	5% of Na₂Sio 3	824	24.84
5	RFA+ Li ₂ Sio ₃		
5	2.5% of Li₂Sio 3	813	24.12



	5% of Li₂Sio 3	817	24.76		
	RFA+ Sio ₂				
6	1% of Sio ₂	821	27.94		
	2.5% of Sio ₂	826	28.46		
	5% of Sio ₂	823	27.14		
	RFA+ Silane Silox	ane (SS)			
7	10% of SS	796	21.20		
	20% of SS	802	21.88		
	RFA+ PVOH + Kaolin				
	2.5% PVOH +	798	20.14		
8	1% Kaolin	790	20.14		
	2.5% PVOH +	810	21.42		
	2% Kaolin	010	21.42		
	RFA+ STEARIC ACID (SA)				
9	(CH3(CH2)16CO)–)			
2	2.5% of SA	794	18.64		
	5% of SA	781	17.13		

Initially to find the best combination the concrete cubes were cured for testing its compressive strength and the results of the cubes prepared with Natural and recycled aggregates are tabulated at table 4.14 and the same has been used as reference for arriving a best combination of treated aggregates in concrete mix and the combination has been listed below.

Table 4.14: Compressive Strength of chemicalcombinations

Sl.no	Sl.no Combinations		Compr Streng (MPa)	essive th
		(kg)	3- Days	7- Days
1	2.5%LS(CA)+2.5%SiO ₂ (FA)	8.010	13.20	23.20
2	5%LS(CA)+2.5%SiO ₂ (FA)	7.940	11.10	20.20
3	2.5%LS(CA)+1%SiO ₂ (FA)	8.100	15.55	24.10
4	1%SiO ₂ (CA)+1%SiO2(FA)	8.040	15.11	22.10
5	20%SS(CA)+1%SiO2(FA)	7.800	11.55	20.65

- From the above a final combination of 2.5% Lithium Silicate treatment on coarse aggregate and 1% Silica dioxide treatment on Fine aggregate found to be the best treatment.
- And important thing is we have used the chemical admixture (super plasticizer) for the combinations.

• The table 4.15 shows the compressive strength results without chemical admixture at two w/c ratio.

Combinations	Weight (kg)	Compressive Strength (MPa)		
	(rg)	3-Days	7-Days	
2.5%LS(CA)+1%SiO2(FA) @ W/C=0.55	8.010	16.00	26.10	
2.5%LS(CA)+1%SiO2(FA) @ W/C=0.50	8.150	20.44	30.22	
FINAL COMBINATIONS FOR CA (2.5% LITHIUM SILICATE)				
FA (1% SILICON DIOXIDE)			-	

Table 4.15: Compressive Strength of chemical combination without chemical admixture

From the above a final combination of 2.5% Lithium Silicate treatment on coarse aggregate and 1% Silica dioxide treatment on Fine aggregate without chemical admixture at w/c is 0.5 found to be the best treatment. For its usage in concrete and on the same to test it mechanical properties of concrete cubes, cylinders and prisms were casted and test for Compressive strength, Split tensile strength, Flexural strength, and the same were compared with that cubes casted with Natural Coarse and fine aggregates and one with Recycled Coarse and fine aggregate. The results on the same are discussed in detail as below.

- From the obtained results compared with that of aggregates prepared with natural aggregates i.e. the value is nearly higher than natural aggregate and up to 13 to 15% than compressive strength of cubes prepared with untreated aggregates.
- From the obtained results compared with that of cylinders prepared with natural aggregates i.e. the value is nearly higher than natural aggregate and up to 12 to 14% than Split tensile strength of Cylinders prepared with untreated aggregates.
- From the obtained results compared with that of Prisms prepared with natural aggregates i.e. the value is nearly higher than natural aggregate and up to 10 to 12% than Flexural strength of prisms prepared with untreated aggregates.



Cubes Tures	Compressive Strength (MPa)			Split Tensile Strength (MPa)	Flexural Strength (MPa)
Cubes Type	3-Days	7-Days	28-Days	28-Days	28-Days
Natural Aggregate (N)	24.44	33.33	40.44	3.30	8.08
M-Sand (M)	24.00	30.00	39.20	3.10	7.50
Recycled Aggregate (R)	17.77	29.80	34.28	3.03	7.33
Treated (T)	20.44	30.22	40.12	3.54	8.25

Table 4.16: Mechanical properties of concrete cubes

Table 4.17: Water absorption of concrete cubes

Cubes Type	Initial weight(kg)	Final weight (kg)	Water Absorption (%age)		
Natural Aggregate	8.550	8.560	0.117		
M-Sand	8.340	8.350	0.119		
Recycled Aggregate	8.050	8.080	0.372		
Treated	8.120	8.130	0.123		

4.8 Durability Study

Durability studies are done on concrete cubes by testing its resistance to acid, sulphate and chloride attacks of the concrete cubes.

Table 4.18: Compressive strength of cubes after 28 days of Chemical curing

Type of mix	Normal	Na ₂ SO ₄	% Strength decrease	NaCl	% Strength decrease	H ₂ SO ₄	% Strength decrease	HCl	% Strength decrease
Natural	41.64	40.16	3.55	40.74	2.16	33.14	20.41	32.96	20.85
M-Sand	39.20	37.74	3.72	38.26	2.40	0.56	22.04	29.86	23.83
Recycled	34.28	32.86	4.14	33.14	3.33	25.64	25.20	25.45	5.76
Treated	40.12	38.64	3.69	39.12	2.49	31.10	224.48	31.56	21.34

Table 4.19: Compressive strength of cubes after 56 days of Chemical curing

Type of mix	Normal	Na ₂ SO ₄	% Strength decrease	NaCl	% Strength decrease	H ₂ SO ₄	% Strength decrease	HCI	% Strength decrease
Natural	41.64	39.54	5.04	39.84	4.32	30.84	25.94	31.56	24.21
M-Sand	39.20	37.16	5.20	37.45	4.46	29.12	25.71	29.36	25.10
Recycled	34.28	31.89	6.97	32.54	5.08	24.89	27.39	24.74	27.83
Treated	40.12	38.12	4.99	38.34	4.44	29.94	25.37	30.01	25.20

5. CONCLUSIONS

This chapter summarizes the assessment and finding of this dissertation work. The conclusions pertaining to comparison of mechanical properties i.e. compressive strength, flexural strength and split tensile strength of chemically treated aggregate with natural and recycled aggregate and also comparisons for durability aspects i.e. chloride, sulphate, acid attack. The fallowing conclusions are made;

- Recycled fine aggregate treated with 1% colloidal silica dioxide can be used as a complete replacement material for sand in cement mortar and in concrete. Similarly Recycled Coarse aggregate treated with 2.5% lithium silicate can be used as a complete replacement material for Natural Coarse aggregate in Concrete.
- Concrete made of treated aggregates has shown very good resistance to acid attacks, Sulphate attacks and chloride attacks, whereas the concrete made of recycled aggregate comparatively failed to resist the above attacks and behaviour of the concrete made of treated aggregates are at par with that of concrete made of conventional aggregates.
- At the time of combinations of chemicals for recycled coarse and fine aggregate it is found that compressive strength of concrete cubes made with treated aggregate with super plasticizers have less compression strength values compared to concrete cubes of treated aggregate without super plasticizer.
- The compressive strength of concrete cube made of treated aggregate shows increase in strength of 13 to 15% when compared with concrete cube made of untreated aggregate and the strength values are nearly equal to natural aggregates.
- The Split tensile strength of concrete cylinders made of treated aggregate shows increase in strength of 12 to 14% when compared with concrete cylinders made of untreated aggregate and the strength values are nearly equal to natural aggregates.
- The Flexural strength of concrete prisms made of treated aggregate shows increase in strength of 10 to 12% when compared with concrete prims made of untreated aggregate and the strength are nearly equal to natural aggregates.

ACKNOWLEDGEMENT

I would like to thank my guide Kiran T, Associate Professor, Dept. of Civil Engg. UVCE, Kishore Ravande, Professor, Dept. of Civil Engg. Osmania University and D. Durga PrasadResearch Scholar, for their valuable advice and technical assistance.

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