# A Comparative Study on High Rise Building for various Geometrical Shapes Subjected to Wind Load of RCC \& Composite Structure using ETABS 

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#### Abstract

In today's modern construction of high rise buildings, lateral loads such as wind load is of major concern. However Steel - Concrete Composite construction has gained wide acceptance worldwide as an alternative to pure steel or pure concrete construction. The review shows that, the composite structures are best suited for high rise buildings compared to that of RCC structures. They offer high stiffness, stability and strength which can be utilized to resist large lateral wind loads and simultaneously support the gravity loading of the structure. In this project, a comparative study has been carried using ETABS 15 software on high rise building for various geometrical shapes subjected to wind load for both RCC and Composite structure. The three geometrical shape of Rectangular, Triangular and a Plus shape are taken with each of similar base plan and same floor to floor height. All the frames are analyzed firstly using RCC frame and then Steel - Concrete Composite frame. The building frame is comprised of $G+15$ storey. By the analysis done using ETABS 15 software the values such as maximum storey displacement, maximum storey shear and maximum storey moment for both Reinforced Concrete and Steel - Concrete Composite structure and comparison has been done for all three geometrical shapes to compare which has more stability and resistance against wind load among all the cases considered.


Key Words: ETABS, RCC, Composite Structure, Wind Load, $\mathrm{G}+15$, Geometrical shapes.

## 1. INTRODUCTION

There has been an increasing demand for construction of tall buildings due to ever-increasing urbanization and need of the population with it. As we increase the height of the building the risk of wind pressure increases. Thus a careful modeling of such wind pressures needs to be done, so as to evaluate the behavior of the structure with a clear perspective of the damage that is expected. Composite structures are generally made up of the interaction of different structural elements and may be developed using either different or similar structural materials. Composite construction has gain wide acceptance because of their many advantages such as they are faster to erect, lighter in weight, better quality control, speedy in terms of construction time, has better ductility than RCC structure and hence superior lateral load resisting behavior. Composite construction also enhances the life expectancy of the structure.

In this project analysis of the different structural models of two different geometrical shapes namely triangular and rectangular having total of 16 storied structure ( $G+15$ ), with both Conventional RCC and Composite Structure and comparing them using ETABS software, to get the optimum and most reliable structural system with the most suitable geometrical shape of the assumed two shapes. A total of Six different cases of the model have been analyzed and designed as a frame structure by the computer application software ETABS, keeping the floor area of each model the same. The design involves load calculations and analyzing the whole structure modeling software and the design method used for analysis is Limit State Method conforming to the Indian Standard Code of Practice.

ETABS is a powerful program that can greatly enhance an engineer's analysis and design capabilities for structures. Part of that power lies in an array of options and features. The other part lies in how simple it is to use. ETABS is a completely integrated system. Embedded beneath the simple, intuitive user interface are very powerful numerical methods, design procedures and international codes, all working from a single comprehensive database. This integration means that you create only one model of the floor system and the vertical and lateral framing systems to analyze and design the entire building. ETABS is very convenient to perform wind loading analysis of the buildings.

## 2. METHODOLOGY

For this study, building of three geometrical shapes of Rectangular, Triangular and Plus shape base have been considered with both Conventional RCC structure and SteelConcrete Composite structure. G+15 storied buildings are modelled using conventional structure of RCC beams, columns \& slabs and composite structure of composite column and steel beam of three different shapes (Rectangular, Triangular and Plus shape). These buildings were given dimensions such that their base area would be same.

Table 1: Description of Case Model Used in Frames

| S. <br> No. | Specifications | Model <br> No. |
| :--- | :--- | :--- |
| $\mathbf{1}$ | G+15 Storied RCC structural model <br> with Rectangular Base Plan | Case 01 |

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| $\mathbf{2}$ | G+15 Storied Composite structural <br> model with Rectangular Base Plan | Case 02 |
| :--- | :--- | :--- |
| $\mathbf{3}$ | G+15 Storied RCC structural model <br> with Triangular Base Plan | Case 03 |
| $\mathbf{4}$ | G+15 Storied Composite structural <br> model with Triangular Base Plan | Case 04 |
| $\mathbf{5}$ | G+15 Storied RCC structural model <br> with Plus Base Plan | Case 05 |
| $\mathbf{6}$ | G+15 Storied Composite structural <br> model with Plus Base Plan | Case 06 |

Now, the model has to be designed for steel - concrete composite structure as well as conventional RCC beam column structure using ETABS software. For the purpose of comparison between the RCC structure and steel-concrete composite structure best efficient and economical section sizes are selected through assessing the maximum bending moment, shear force, maximum deflection, and nodal displacement of column due to load combination. The focus is on steel-concrete structural members, their connections and the effects of their interactions and reliability of the composite structure with general loading and wind loading applied on the structure over conventional reinforced concrete structure.

### 2.1 Assumptions for the modelling-

- Only the main block of the building is considered. The staircase are not considered in the design procedure.
- The beams are resting centrally on the column so as to avoid the conditions of eccentricity. This is achieved automatically in ETABS.
- For all structural elements, M25 and Fe415 grade of concrete and steel are used.
- The footing are not designed. Supports are assigned in the form of fixed supports.


Fig. 1 - ETABS generated rendered model for cases 01, 02, $03,04,05,06$

Table 2 - Member Properties \& Specifications for the Model

| S. <br> No. | Specification | Size |
| :--- | :--- | :--- |
| $\mathbf{1}$ | Base Area | 200 sq. m. (as per <br> plan) |


| 2 | Floor to floor height |  |  | 3.5 m |
| :---: | :---: | :---: | :---: | :---: |
| 3 | Total height of the building$(\mathrm{G}+15)$ |  |  | 56m |
| 4 | Slab Thickness |  |  | 150 mm |
| 5 | Type of structure |  |  | Conventional RCC \& Composite |
| 6 | Soil type (as per 1893:2002) |  |  | Medium |
| 7 | Importance Factor |  |  | 1 |
| 8 | Seismic Zone |  |  | Zone II |
| 9 | Grade of Concrete |  |  | M25 |
| 10 | Grade of Steel |  |  | M415 |
| 11 | Beam Size |  |  | 300 mm X 450mm |
| 12 | Column Size |  |  | 500 mm X 500 mm |
| 13 | Loads Applied | D.L. | Deal <br> Load | calculated as per selfweight |
|  |  |  | Floor Finish | $1 \mathrm{kN} / \mathrm{m} 2$ |
|  |  | L.L. | Live <br> Load | $2.5 \mathrm{kN} / \mathrm{m} 2$ |
|  |  | W.L. | Wind Load | calculated as per IS 875 part3 |
| 14 | Load Combination |  |  | 1.2(DL + LL + WL) |

### 2.2 Section Properties -

The built-up area considered are taken equal for all plans of different shaped frames, with base plan are of 200 sq.m. The floor to floor height is taken as 3.5 meter making the total height of the structure 56 meter and the whole analysis has been carried out using ETABS software. Assigning the material properties for concrete grade M20 and Fe415, then assigning the section properties of beam of size $300 \mathrm{~mm} x$ 450 mm and column size of $500 \mathrm{~mm} \times 500 \mathrm{~mm}$ with concrete grade of M20 and steel grade of Fe 415 which are same for all frame structural cases considered.

The cross-section properties of the beam that are taken in the ETABS software are as shown in the figure below. RCC beam of size $300 \mathrm{~mm} \times 450 \mathrm{~mm}$ for Conventional RCC frame structure and I-section (ISHB400) hot rolled beam for Composite structure for all three shaped model structure.

The cross-section properties of the Column that are taken in the ETABS software are as shown in the figure below. RCC Column of size $500 \mathrm{~mm} \times 500 \mathrm{~mm}$ for Conventional RCC frame structure and Tabular section filled with concrete column section for Composite structure for all three shaped model structure.

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## 3. RESULT AND DISCUSSION

### 3.1 Comparison of Maximum Storey Displacement -

The Maximum displacement (along Y-axis) for the each storey as per the output generated from the ETABS Software is given below in table.

Table 3 - Storey Displacement for Rectangular Frame

| Storey Maximum Storey displacement (mm) |  |  |
| :---: | :---: | :---: |
| Storey | Case 01 | Case 02 |
|  | RCC-Rectangular | Composite-Rectangular |
| Sixteen | 315.614 | 310.783 |
| Fifteen | 310.178 | 305.981 |
| Fourteen | 303.111 | 299.615 |
| Thirteen | 294.162 | 291.351 |
| Twelve | 283.317 | 281.120 |
| Eleven | 270.616 | 268.936 |
| Ten | 256.111 | 254.839 |
| Nine | 239.865 | 238.883 |
| Eight | 221.949 | 221.131 |
| Seven | 202.450 | 201.660 |
| Six | 181.469 | 180.553 |
| Five | 159.112 | 157.889 |
| Four | 135.474 | 133.703 |
| Three | 110.546 | 107.863 |
| Two | 83.813 | 79.681 |
| One | 52.074 | 46.566 |

Table 4 - Storey Displacement for Triangular Frame

| Storey Maximum Storey displacement (mm) |  |  |
| :---: | :---: | :---: |
| Storey | Case 03 | Case 04 |
|  | RCC-Triangular | Composite-Triangular |
| Sixteen | 545.389 | 518.385 |
| Fifteen | 535.980 | 511.302 |
| Fourteen | 523.931 | 501.667 |
| Thirteen | 508.652 | 488.819 |
| Twelve | 490.090 | 472.602 |
| Eleven | 468.293 | 453.011 |
| Ten | 443.347 | 430.100 |
| Nine | 415.354 | 403.947 |
| Eight | 384.434 | 374.649 |
| Seven | 350.735 | 342.323 |
| Six | 314.433 | 307.097 |
| Five | 275.708 | 269.083 |
| Four | 234.714 | 228.305 |
| Three | 191.394 | 184.472 |
| Two | 144.771 | 136.314 |
| One | 88.998 | 79.249 |

Table 5 - Storey Displacement for Plus Frame

| Storey Maximum Storey displacement (mm) |  |  |
| :---: | :---: | :---: |
| Storey | Case 05 | Case 06 |
|  | RCC-Plus | Composite-Plus |
| Sixteen | 254.731 | 250.097 |


| Fifteen | 250.791 | 246.595 |
| :---: | :---: | :---: |
| Fourteen | 245.500 | 241.793 |
| Thirteen | 238.652 | 235.424 |
| Twelve | 230.237 | 227.439 |
| Eleven | 220.286 | 217.852 |
| Ten | 208.839 | 206.694 |
| Nine | 195.944 | 194.009 |
| Eight | 181.654 | 179.844 |
| Seven | 166.038 | 164.259 |
| Six | 149.175 | 147.323 |
| Five | 131.150 | 129.101 |
| Four | 112.043 | 109.629 |
| Three | 91.869 | 88.818 |
| Two | 70.253 | 66.119 |
| One | 44.477 | 39.237 |

Graph 1 - Comparison of Maximum Storey Displacement for all cases


From the comparison of maximum storey displacement for Rectangular, Triangular \& Plus plan the following results has been derived

- The value of maximum storey displacement in Composite Structure get decreased by about $1 \%$ for rectangular plan as compared to RCC Structure.
- The value of maximum storey displacement in Composite Structure get decreased by about $5 \%$ for triangular plan as compared to RCC Structure.
- The value of maximum storey displacement in Composite Structure get decreased by about approx. 2\% for Plus plan as compared to RCC Structure.
- The value of maximum storey displacement in top floor get increased by about average $42 \%$ for RCC model and as we change the structure from Rectangular to Plus, the value of maximum storey displacement in top floor decreased by around 23\% for RCC Structure model.

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- The value of maximum storey displacement in top floor get increased by about average $40 \%$ for RCC model and as we change the structure from Rectangular to Plus, the value of maximum storey displacement in top floor get Decreased by about 24\% for Composite Structure model.


### 3.2 Comparison of Maximum Storey Shear (in kN) -

Table 6 - Storey Shear for Rectangular Frame

| Maximum Storey Shear (kN) |  |  |
| :---: | :---: | :---: |
| Storey | Case 01 | Case 02 |
|  | RCC-Rectangular | Composite- <br> Rectangular |
| Sixteen | 4033.297 | 2601.662 |
| Fifteen | 8066.594 | 5203.323 |
| Fourteen | 12099.891 | 7804.985 |
| Thirteen | 16133.188 | 10406.647 |
| Twelve | 20166.484 | 13008.308 |
| Eleven | 24199.781 | 15609.970 |
| Ten | 28233.078 | 18211.632 |
| Nine | 32266.375 | 20813.294 |
| Eight | 36299.672 | 23414.955 |
| Seven | 40332.969 | 26016.617 |
| Six | 44366.266 | 28618.279 |
| Five | 48399.563 | 31219.940 |
| Four | 52432.860 | 33821.602 |
| Three | 56466.156 | 36423.264 |
| Two | 60499.453 | 39024.925 |
| One | 64532.750 | 41626.587 |

Table 7 - Storey Shear for Triangular Frame

| Maximum Storey Shear (kN) |  |  |
| :---: | :---: | :---: |
| Storey | Case 03 | Case 04 |
|  | RCC-Triangular | Composite- <br> Triangular |
| Sixteen | 3664.510 | 1370.139 |
| Fifteen | 7329.019 | 1240.278 |
| Fourteen | 10993.529 | 4110.417 |
| Thirteen | 14658.038 | 5480.555 |
| Twelve | 18322.547 | 6850.694 |
| Eleven | 21987.057 | 8220.833 |
| Ten | 25651.566 | 9590.972 |
| Nine | 29316.076 | 10961.111 |
| Eight | 32980.585 | 12331.249 |
| Seven | 36645.095 | 13701.388 |
| Six | 40309.604 | 15071.527 |
| Five | 43974.114 | 16441.666 |
| Four | 47638.623 | 17811.805 |
| Three | 51303.133 | 19181.944 |
| Two | 54967.642 | 20552.082 |
| One | 58632.152 | 21922.221 |

Table 8 - Storey Shear for Plus Frame

| Maximum Storey Shear (kN) |  |  |
| :---: | :---: | :---: |
| Storey | Case 05 | Case 06 |
|  | RCC-Plus | Composite-Plus |
| Sixteen | 1921.973 | 1668.110 |
| Fifteen | 3843.946 | 3336.220 |
| Fourteen | 5765.919 | 5004.330 |
| Thirteen | 7687.893 | 6672.439 |
| Twelve | 9609.866 | 8340.549 |
| Eleven | 11531.839 | 10008.659 |
| Ten | 13453.812 | 11676.769 |
| Nine | 15375.785 | 13344.879 |
| Eight | 17297.758 | 15012.989 |
| Seven | 19219.731 | 16681.098 |
| Six | 21141.705 | 18349.208 |
| Five | 23063.678 | 20017.318 |
| Four | 24985.651 | 21685.428 |
| Three | 26907.624 | 23353.538 |
| Two | 28829.597 | 25021.648 |
| One | 30751.570 | 26689.757 |

Graph 2 - Comparison of Maximum Storey Shear for all cases


From the comparison of maximum storey Shear for Rectangular, Triangular \& Plus plan the following results has been derived

- As we change the structure from conventional RCC to composite, the value of maximum storey displacement in top floor get decreased by about 35.50\% for rectangular plan.
- As we change the structure from conventional RCC to composite, the value of maximum storey displacement in top floor get decreased by about $62.61 \%$ for triangular plan.
- As we change the structure from conventional RCC to composite, the value of maximum storey displacement in top floor get decreased by about $13.21 \%$ for Plus plan.

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- As we change the structure from Rectangular to Triangular, the value of maximum storey displacement in top floor get decreased by about 9\% for RCC model and as we change the structure from Rectangular to Plus, the value of maximum storey displacement in top floor get decreased by about 52\% for RCC Structure model.
- As we change the structure from Rectangular to Triangular, the value of maximum storey displacement in top floor get decreased by about $47 \%$ for Composite model and as we change the structure from Rectangular to Plus, the value of maximum storey displacement in top floor get decreased by about $36 \%$ for Composite model.


### 3.3 Comparison of Maximum Storey Moment ( $\mathbf{k N}$-m) -

Table 6 - Storey Moment for Rectangular Frame

| Maximum Storey Moment (kN-m) |  |  |
| :---: | :---: | :---: |
| Storey | Case 01 | Case 02 |
|  | RCC-Rectangular | Composite- <br> Rectangular |
| Sixteen | 20550.052 | 13391.876 |
| Fifteen | 41862.899 | 27546.547 |
| Fourteen | 63930.339 | 42455.811 |
| Thirteen | 86741.215 | 58108.511 |
| Twelve | 110284.144 | 74493.264 |
| Eleven | 134547.835 | 91598.779 |
| Ten | 159521.081 | 109413.849 |
| Nine | 185192.424 | 127927.016 |
| Eight | 211544.788 | 147121.204 |
| Seven | 238556.624 | 166974.864 |
| Six | 266206.331 | 187466.395 |
| Five | 294470.166 | 208572.054 |
| Four | 323320.715 | 230264.427 |
| Three | 352779.727 | 252515.263 |
| Two | 382689.353 | 275316.713 |
| One | 401000.241 | 298668.760 |

Table 7 - Storey Moment for Triangular Frame

| Maximum Storey Moment (kN-m) |  |  |
| :---: | :---: | :---: |
| Storey | Case 03 | Case 04 |
|  | RCC-Triangular | Composite- <br> Triangular |
| Sixteen | 25825.711 | 9565.601 |
| Fifteen | 52414.215 | 19893.995 |
| Fourteen | 79757.314 | 30976.984 |
| Thirteen | 107843.848 | 42803.408 |
| Twelve | 136662.436 | 55361.886 |
| Eleven | 166201.784 | 68641.125 |
| Ten | 196450.689 | 82629.920 |
| Nine | 227397.690 | 97316.811 |
| Eight | 259025.712 | 112684.723 |
| Seven | 291313.206 | 128712.107 |
| Six | 324238.572 | 145377.363 |


| Five | 357778.065 | 162656.746 |
| :---: | :---: | :---: |
| Four | 391904.272 | 180522.844 |
| Three | 426588.943 | 198947.404 |
| Two | 461824.226 | 217922.578 |
| One | 497610.108 | 237448.350 |

Table 8 - Storey Moment for Plus Frame

| Maximum Storey Moment (kN-m) |  |  |
| :---: | :---: | :---: |
| Storey | Case 05 | Case 06 |
|  | RCC-Plus | Composite-Plus |
| Sixteen | 15645.845 | 12894.392 |
| Fifteen | 30359.527 | 26551.577 |
| Fourteen | 46675.282 | 40963.357 |
| Thirteen | 63734.471 | 56118.572 |
| Twelve | 81525.715 | 72005.841 |
| Eleven | 100037.720 | 88613.871 |
| Ten | 119259.280 | 105931.456 |
| Nine | 139178.937 | 123947.138 |
| Eight | 159779.615 | 142643.842 |
| Seven | 181039.765 | 162000.017 |
| Six | 202937.786 | 181994.064 |
| Five | 225449.935 | 202602.238 |
| Four | 248548.798 | 223797.126 |
| Three | 272206.125 | 245550.478 |
| Two | 296414.065 | 267854.442 |
| One | 321172.602 | 290709.005 |

Graph 3 - Comparison of Maximum Storey Moment


From the comparison of maximum storey Moment for Rectangular, Triangular \& Plus plan the following results has been derived

- As we change the structure from conventional RCC to composite, the value of maximum storey displacement in top floor get decreased by about average 35\% for rectangular plan respectively.
- As we change the structure from conventional RCC to composite, the value of maximum storey displacement in top floor get decreased by about average $63 \%$ for triangular plan respectively.
- As we change the structure from conventional RCC to composite, the value of maximum storey displacement in top floor get decreased by about average 18\% for Plus plan respectively.
- As we change the structure from Rectangular to Triangular, the value of maximum storey displacement in top floor get increased by about 20\% for RCC model and as we change the structure from Rectangular to Plus, the value of maximum storey displacement in top floor get decreased by about 31\% for RCC Structure model.
- As we change the structure from Rectangular to Triangular, the value of maximum storey displacement in top floor get decreased by about 29\% for Composite model and as we change the structure from Rectangular to Plus, the value of maximum storey displacement in top floor get decreased by about $4 \%$ for Composite Structure model


## 4. CONCLUSIONS

In all the cases considered the values of storey displacements are within permissible limits as per IS code limits.

It is safe to conclude that case-06 with Plus plan of Composite frame structure gives best result from all the cases that has been compared and is more stable than other cases.

As we chamfer the edged of rectangular or square plan frame structure the resistance to the lateral wind load increases and with the help of Steel - Concrete Composite structure the stability of the structure can be further increased.

The size of the steel beams of Steel-Concrete Composite frame structure from RCC frame structure reduces by about $25 \%$ approximately. Thus dead load of the composite structure is less as compared to RCC frame structure, which gives economical foundation design.

Also as time required for construction of composite structures is less compared to that of RCC structures as no formwork is required. Thus Steel-Concrete Composite structures are more economical in case of high rise buildings.

Steel-Concrete Composite frame follows strong column weak beam behaviour, as hinges are formed in beam element rather than column element.

Composite columns are also used widely in practice to resist predominantly compressive loading and appear in different form including concrete filled section, recently using high strength high performance concrete.

The further development of steel framed buildings depends largely on the use of composite construction as its construction is speedy and reduces the erection time.

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