

SEISMIC ANALYSIS OF G+6 FRAME BUILDING USING ETABS

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Abstract – Seismic Analysis of any type of structure is an important consideration while working in high earthquake prone areas. With the help of seismic analysis, the structure can be designed and constructed to withstand the high lateral moment if earth crust during an earthquake. Any type of basic or highly advanced structure which may be under static or dynamic conditions can be evaluated by using ETABS.ETABS is a coordinated and productive tool for analysis and designs which range from simple 2D frames to modern high rises which makes it one of the best structural software for building software for building systems. A G+6 storey building of different shaped frame structures is considered for this study.

Key Words: ETABS-2015, Seismic Analysis, Time Period, Story Drift, Base Shear, Story Stiffness.

1. INTRODUCTION

The main aim is to generate and perform linear static analysis of 4 different shapes of structure: Rectangular, H-Shaped, C-Shaped, L-Shaped and to compare their results for Time period, Base shear, Story drift, Story stiffness. It is expected that these structures will sustain all the loads and deformations of normal construction and have adequate durability and resistance to seismic effects.

Generally structural designers are prone to use linear static analysis which is also known as first order analysis to compute the response of a structure such as design force, moments and displacements resulting from loads acting on the structure. In case of first order analysis only the small deflections moments are considered. Additional effect due to the deformation of the structure under vertical loads are neglected.

2. MODELLING OF RCC FRAME

We have considered a 3D RCC frame with the dimensions for 4 different shapes. These shapes have same area. The 4 shapes are as follows:

- 1. Rectangular plan
- 2. H-shaped plan
- 3. C-shaped plan
- 4. L-shaped plan



Fig 1: Plan (a) Rectangular shaped (b) H-shaped (c) C- shaped (d) L-shaped of the building.

Table 1	1
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Building Description And Material Specification			
Number of storey	6		
Support condition	Fixed		
Storey height	3m		
Height of building	18m		
Grade of concrete			
Beam	25 N/mm2		
Column	30 N/mm2		
Slab	20 N/mm2		
Grade of steel	Fe500		
Size of column	450mm X 450mm		
Size of beam	250mm X 400mm		
Thickness of wall	230mm		
Thickness of slab	150mm		
Density of concrete	25KN/m3		
Density of brick walls	22.5KN/m3		
consider			

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3. LOADING

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The structures are acted upon by different loads such as dead load, live load, earthquake load.

- 1. Self-weight of structure comprises of weight of the beam, column and slab of the structure.
- 2. Dead load of the structure consists of wall load, parapet wall load and floor load according to IS 875 part 1.
 - I. Wall load = (weight unit of brick masonry * thickness of wall * height of wall)
 - = 22.5kN/m3 * 0.23m * 3m
 - = 22.5 kN/m3 $\cdot 0.25$ m $\cdot 5$ = 15.525 kN/m3.
- II. Wall load (parapet wall at top floor)
 - = (weight unit of brick masonry * thickness of wall * height of wall)
 - = 22.5kN/m3 * 0.115m * 1.5m
 - = 3.88KN/m.
- 3. Live load: It consists of floor load which is taken as 3KN/m2 and roof load as 1.5kN/m2 according to IS 875 part-2.
- 4. Seismic load: The different seismic parameters are taken as follows, IS 1893 part 1 : 2002
 - 1. Seismic zone : II (Z=0.10)
 - 2. Soil type : II
 - 3. Importance factor : 1
 - 4. Response reduction factor: 3
 - 5. Dumping : 5%

4. LOADING COMBINATION

The structural systems were subjected following load combinations as per provisions of **IS 1893-2002 (Part I)**, Clause 6.3.1, that deals with "Criteria for Earthquake Resistant Design of Structures".

Analysis is carried out for gravity loads using partial safety factor as 1.5. The following Loads have been considered in the structural analysis and design as per IS code 456-2000.

- 1. D-Dead load
- 2. LL-live load
- 3. EQ- Earthquake

Table 2

LOAD COMBINATIONS	REMARKS
1.5(DL+LL)	* DL - Dead load of the
1.5(DL ± EQX)	structure
1.5(DL ± EQY)	* LL - Live load of the
(0.9DL ± 1.5EQX)	structure
(0.9DL ± 1.5EQY)	* EQX - Earthquake load
1.2(DL+LL±EQX)	along X direction
1.2(DL+LL±EQY)	* EQY - Earthquake load
	along Y direction 1

5. MODELLING IN ETABS



Fig 2: 3-D view of 6-storeys Rectangular shape building



Fig 3: 3-D view of 6-storeys H-shape building



Fig 4: 3-D view of 6-storeys C-shape building



Fig 5: 3-D view of 6-storeys L-shape building

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6. RESULTS AND DISCUSSIONS

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1. Time Period: It is the time needed for one complete cycle of vibration to pass a given point. As the frequency of a wave increases, the time period of the wave decreases. Frequency and time period are in a reciprocal relationship that can be expressed mathematically as, T = 1/f or as, f = 1/T. Table 3 represents the fundamental period of first three modes of all structure systems.

Tab	le	3
		-

STRUCTURAL SYSTEM	MODE 1	MODE	MODE
		2	3
Rectangle shape	0.174	0.173	0.173
H shape	0.126	0.126	0.126
L shape	0.108	0.104	0.071
C shape	0.098	0.098	0.098



Chart 1: Time Period

The result of structural section L is having variation in values in three modes. The structural system like rectangular section, H section, C section is having the values same for first three modes.

2. Base Shear: It stands an assessment of the extreme predictable lateral force that will arise at the base of a building due to applied loads. Variation of base shear for different structural systems observed and the results for seismic zone II in EQX, EQY are tabulated in below table.

Та	bl	le	4	
1 0	~	-		

Structural Shape	EQX	EQY
Rectangular shape	3200.27	3200.27
H shape	3112.55	3112.55
L shape	1860.9404	1860.9404
C shape	2230.3828	2230.3828



Chart 2: Base Shear

The result of rectangular shaped structure can able to resist maximum base shear compare to other three systems considered in study.

L shape structure resist a minimum base shear compared to other three systems considered in study.

3. Storey Drift: It is defined as the ratio of displacement of two successive floors to height of that floor. It is unit less. The permissible limit storey drift of any based IS 1893 – 2002.

Table 4	(a)·	Story	Drift a	long X	direction
I able 4	aj.	Story	DINCA	iong A	unection

STOR EY	RECTANGLE SHAPE	H SHAPE	L SHAPE	C SHAPE
6	0.000008	1.00E-06	0.000014	0.000002
5	0.000007	1.00E-06	0.000017	0.000002
4	0.000006	1.00E-06	0.000017	0.000002
3	0.000004	1.00E-06	0.000016	0.000001
2	0.000003	1.00E-06	0.000015	0.000001
1	0.000002	3.66E-07	0.000012	4.26E-07
BASE	0	0.00E+0	0	0



Chart 3(a): Storey Drift along X- direction

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The results of structural system L section can able to resist maximum storey drift compared to other three structures. H shaped structure resist a Minimum storey drift compared to the other three structures.

Table 3(b): Storey Drift along Y-direction

STOR EY	<u>RECTANGL</u> <u>E SHAPE</u>	<u>H SHAPE</u>	<u>L SHAPE</u>	<u>C SHAPE</u>
<u>6</u>	<u>0.000008</u>	<u>1.00E-06</u>	<u>0.000014</u>	<u>0.000003</u>
<u>5</u>	<u>0.000007</u>	<u>1.00E-06</u>	<u>0.000017</u>	<u>0.000003</u>
<u>4</u>	<u>0.000006</u>	<u>1.00E-06</u>	<u>0.000017</u>	<u>0.000003</u>
<u>3</u>	<u>0.000004</u>	<u>1.00E-06</u>	<u>0.000016</u>	<u>0.000002</u>
<u>2</u>	<u>0.000003</u>	<u>1.00E-06</u>	<u>0.000015</u>	<u>0.000001</u>
1	<u>0.000002</u>	<u>3.66E-07</u>	<u>0.000012</u>	<u>0.000001</u>
BASE	<u>0</u>	<u>0.00E+0</u>	<u>0</u>	<u>0</u>



Chart 3(b): Storey Drift along Y-direction

The results of structural system L section can able to resist maximum storey drift compared to other three structures. H shaped structure resist a Minimum storey drift compared to the other three structures.

4. Storey Stiffness: The storey stiffness of a storey is a generally defined has ratio of to storey drift. storey shear

Table 4(a): Storey Stiffness along X-direction

STOR	RECTANG	H SHAPE	L	C SHAPE
EY	LE		SHAPE	
	SHAPE			
6	10214568	48002694	232469	31105253
		7	57	4
5	20024831	92498855	324081	54389320
		0	46	6
4	30203010	13113731	389110	76350445
		14	94	9

L



Chart 4(a): Storey Stiffness along X-direction

Storey stiffness value is maximum in X direction. H shape structure is increases as compared to all other shapes like rectangle shape, L shape, C shape.

	DDOT			
STOREY	RECTANG	H SHAPE	L SHAPE	C SHAPE
	LE			
	SHAPE			
6	10189065	48002694	2324695	16763335
	3	7	7	4
5	20017632	92498855	3240814	31432671
	5	0	6	0
4	30203455	13113731	3891109	44252278
	0	14	4	3
3	45814791	18769980	4483141	63308049
	7	99	7	4
2	75007356	29361404	4968482	97637791
	0	59	9	1
1	12165228	48396955	6162444	14728630
	71	11	6	79



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Storey stiffness value is maximum in Y direction. H shape structure is increases as compared to all other shapes like rectangle shape, L shape, C shape.



Fig 6: 3D view of Rectangular Section after Seismic analysis



Fig 7: 3D view of H-Section after Seismic Analysis







Fig 9: 3D view of L-Section after Seismic Analysis

7. CONCLUSIONS

1. Analysis was done by using ETABS software successfully verified manually as per IS456:2000 and IS 1893-2002 (Part I), Clause 6.3.1, that deals with "Criteria for Earthquake Resistant Design of Structures".

2. Calculation by both manual work as well as software analysis gives almost same result.

3. Usage of ETABS software minimizes the required for analysis design.

4. The plan configurations of structure has significant impact on the seismic response of structure in terms of displacement, story drift, story shear, time period.

5. As the result, in X and y direction the story displacement/drift is increased in L structure as compared with other structure like rectangular, H – shape, and C shape and it is observed that the storey drift for the stories are found to be within the permissible limits.

6. In X and Y direction the story stiffness is increased in L structure as compared with other structure like rectangular, H –shape, and C shape.

7. Base shear in X and Y direction is same for all structures like rectangular, H –shape, and C shape, with increase in load.

8. Story stiffness valve is maximum in X and Y direction of H shape structure compared to other structure.

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