

Comparative Study of Flat Slab and Conventional Slab Structure with and without shear walls Using ETABS

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Abstract - In today's construction activity the use of flat slab is quite common which enhances the weight reduction, speed up construction, and economical. Similarly from the beginning conventional slab has go it place in providing features like more stiffness, higher load carrying capacity, safe and economical also. As the advancement era began practice of flat slab becomes quite common. In the present dissertation work a G+14 commercial multistoried building having flat slab and conventional slab has been analyzed for the parameters like base shear, storey drift, Storey stiffness, and displacement. The performance and behavior of both the structures in seismic zones II & V of India has been studied.

Displacement of industrial and commercial structure constructed using flat slab system is more than the conventional slab system. Here we can say that flat slab with shear wall gives better displacement resisting. With the increase in height of structure displacement is also goes on increasing. It is seen that story drift is maximum for the conventional slab compared to flat slab and very less for the flat slab with shear wall. Story stiffness of conventional slab building is stiffer than Flat slab building. As the story no decreases stiffness goes on increasing.

Key Words: Flat slab, drop, conventional slab, Shear wall, ETABS.

1. INTRODUCTION

From last two decades there is a high increase in the high rise buildings and modern trend is towards high rise structures. In tall buildings with increase in height lateral loads have prime consideration. From the effect of gravity resulting most common loads are dead load, live load and snow load. Buildings are also subjected to lateral loads caused by wind and earthquake. Due to the lateral loads develop high stresses, produce sway movement or vibrations.

Flat slab are used to avoid the beam-column clogging, and it is very economical. Flat slabs directly transfer the loads to columns without beams. But flat slabs are not efficient in transfer the lateral loads. Punching shear strength around the column-slab connections always possess a problem. Punching shear is a type of failure of reinforced concrete slabs subjected to high localized forces. When the total shear force exceeds the shear resistance of the slab, the slab will be pushed down around the column is termed as punching shear in flat slabs. This results in the column breaking through the portion of the surrounding slab. As a solution of

seismic load resistance, time and cost effective construction shear walls are most effective one method.

Flat plate construction is widely used in residential, office and industrial buildings in many parts of the world. The main advantage of this construction is the faster construction compared to mushroom and ribbed slabs. Generally, slabs are supported by beams and these beams are supported by columns. Beam reduces available net clear ceiling height. Sometimes beams are avoided and slabs are directly supported by columns. This type of construction provides aesthetical appearance also. Those slabs which are directly supported by columns are called as flat slabs. Flat slab also referred as beamless slab, it is the directly connected by columns without beams. Due to the advantages of flat slabs over other reinforced concrete floor system engineers are mostly used in construction works.

The main disadvantages of flat slab systems are; they are not suitable for supporting brittle (masonry) partitions, higher slab thickness, Chance for progressive collapse is more in flat slab due to the punching shear failure, in flat slabs, the middle strip deflection may be critical.

1.1 CONVENTIONAL BEAM SLAB SYSTEM

Conventional slab is a slab supported in all 4 edges by beams. The loads are transferred by the beams to the columns. Conventional slab system is generally used in residential buildings and in small construction. The main advantage of this conventional slab system is, we can design for a maximum span and maximum load by increasing the depth of the beams and cross section of the columns without any significant increase in the depth of the slab.

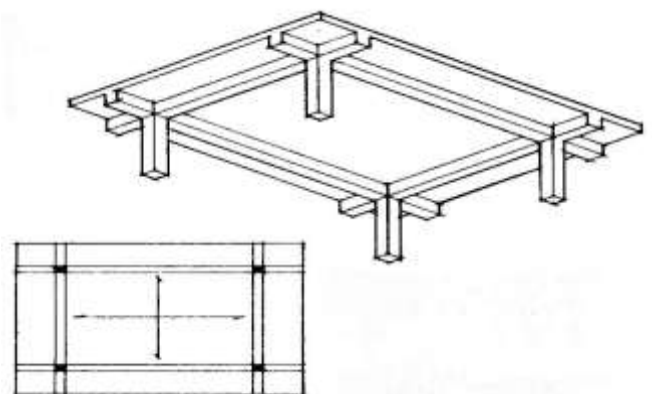


Figure: 1 CONVENTIONAL SLAB

1.2 FLAT SLAB SYSTEM

A reinforced concrete flat slab, also called as beamless slab, is a slab supported directly by columns without beams. A part of the slab bounded on each of the four sides by center line of column is called panel. The thickened portion i.e. the projection below the slab is called drop or drop panel. Flat slab is mainly used in commercial buildings where the aesthetic view is more important and for the ease of the construction of formwork. Flat slab is a reinforced concrete slab supported directly by concrete columns without the use of beams. Flat slab is defined as one sided or two-sided support system with shear load of the slab being concentrated on the supporting columns and a square slab called drop panels.

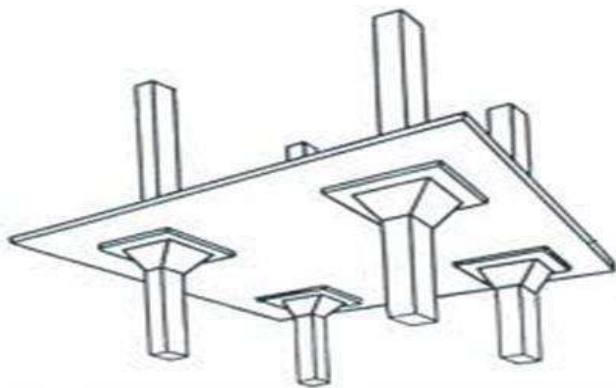


Figure: 2 Flat slab

1.3 Shear wall as a bracing method

Shear walls are the vertical elements to resist the horizontal force in a structure. Shear walls directly resist the lateral force along the length of the wall. By properly detailed longitudinal and transverse reinforcement can achieve the necessary strength to avoid the structural damage under earthquakes. Lateral forces are derived from winds and earthquakes that are applied horizontally to the building.

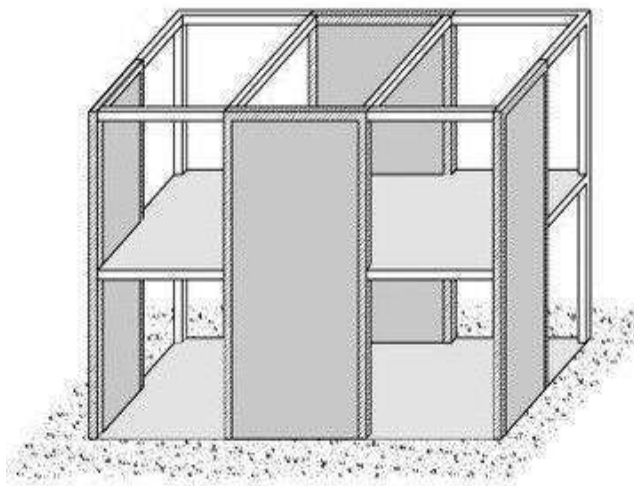


Figure: 3 Building with shear wall

2. LITERATURE REVIEW

Lan N Robertson (1997) [1] In this study the analysis of flat slab structures subjected to combined lateral and gravity loads. Using a three dimensional model, analysis of a flat slab building can have done when it subjected to vertical and lateral loads which includes both slab column frame elements and the lateral framing system (shear wall) if present. This study reviews two structural analysis models and compares them to experimental test results. A two-beam analytical model more accurately predicts the test results with respect to slab moment distribution and lateral drift. Three dimensional analysis done by ETABS computer program. These models assume a uniform slab effective width coefficient and constant cracking factor for an entire span. The analytical models were unable to reproduce the slab flexural moment distribution observed in test specimen at either 0.5 or 1.5 % drift levels. By replacing the single beam element with two-beam elements connected at the point of contra flexure, the difference between cracking in the positive and negative moment regions was incorporated in to the mode.

M A Rahman (2012) [2] He conducted a study on effect of openings in shear wall on seismic response of structures. In this paper, finite element modeling in analyzing and exploring the behaviour of shear wall with opening under seismic load actions, an attempt is made to apply the finite element modeling. A shear wall in a building contains many openings due to functional requirements such as doors, windows and other openings. This study is carried out using linear elastic analysis with the help of software ETABS under the earthquake loads in equivalent static analysis. This study reveals that, the size of the openings as well as their locations in shear walls, if will affect the stiffness as well as seismic responses of structure. If the area of openings more, the displacement increases with increasing storey level. Thickening wall around the door openings are more effective than that of window opening as far as displacements in concerned at top most storey level.

Lakshmi K O (2014) [4] In this journal find the effect of shear wall location in buildings subjected to seismic loads. A symmetric sixteen story residential building considered for the analysis. The finite element analysis software ETABS is used to create the 3-D model and run the analysis by pushover method. Eight different models were considered. Due to the seismic ground motion at the base of the structure base shear is maximum. Maximum reduction in displacement is obtained for frame with core and corner shear wall.

3. OBJECTIVES

1. To obtain the most effective structure to resist the lateral loads.
2. To identify the most vulnerable building among the models considered for seismic action.

3. Efficiency of concrete structures with and without shear wall with respect the story displacement, drift and Storey stiffness, overturning moment, time period.

4. The effect of shear wall on behavior of concrete structures are summarized using the obtained results, by concluding the variation of results in structures.

4. METHODOLOGY

Following methodology is adopted to analyses.

1. RC concrete structure is considered for the study having 15 stories of height 45 m each floors is considered as 3 m height.
2. The regular concrete moment resisting frame of square plan is considered as base or reference model.
3. With reference to base model, Flat slab model are studied and compared with shear wall for all the models for seismic.
4. In order to get consistent results, the floor height is kept constant for all structures.
5. To understand the behavior under lateral loads applied as per IS 1893: 2002 are used respectively.
6. Based on the results and responses from earthquake loads applied, conclusion are made.

5. PARAMETERS CONSIDERED FOR MODELLING

Table- 1 Preliminary Data for 15-story Conventional slab building.

SL.NO	PARAMETERS	
1	Length in X-direction	35m
2	Length in Y-direction	35m
3	Floor to floor height	3m
4	No of stories	15
5	Total height of the building	45m
6	Slab thickness	150mm
7	Grade of concrete	M30
8	Grade of steel	HYSD 415,500
9	Wall size	300mm
10	Column size	500mmX500mm
11	Beam size	300mmx600mm
12	Live load in Floors	2kN/m ²
13	Live load in Terrace	1.5kN/m ²
14	Floor finish	1.5kN/m ²

Table -2: Preliminary Data for 15-story Flat slab building.

SL.NO	PARAMETERS	
1	Length in X-direction	35m
2	Length in Y-direction	35m
3	Floor to floor height	3m
4	No of stories	15
5	Total height of the building	45m
6	Slab thickness	150mm

7	Drop thickness	200mm thick
8	Grade of concrete	M30
9	Grade of steel	HYSD 415,500
10	Wall size	300mm
11	Column size	500mmX500mm
12	Beam size	300mmx300mm
13	Shear wall thickness	150mm
14	Live load in Floors	2kN/m ²
15	Live load in Terrace	1.5kN/m ²
16	Floor finish	1.5kN/m ²

Table -3: Preliminary Data for Seismic Load Parameters

SL.No	Seismic load parameters	Zone 2-5
1	Zone factor	0.16-.36
2	Response reduction factor	5
3	Importance factor	1.5
4	Type of soil strata	2(Medium)
5	Damping	5%

Table -4: Load Combinations

Type	Design Load Combinations
Gravity analysis	1.5 (Dead Load + Live Load)
Equivalent Static Analysis	1.2 (Dead Load + Live Load + EQX)
	1.2 (Dead Load + Live Load - EQX)
	1.2 (Dead Load + Live Load + EQY)
	1.2 (Dead Load + Live Load - EQY)
	1.5 (Dead Load + EQX)
	1.5 (Dead Load - EQX)
	1.5 (Dead Load + EQY)
	1.5 (Dead Load - EQY)
	0.9 (Dead Load + EQX)
	0.9 (Dead Load - EQX)
	0.9 (Dead Load + EQY)
	0.9 (Dead Load - EQY)

6. Models considered.

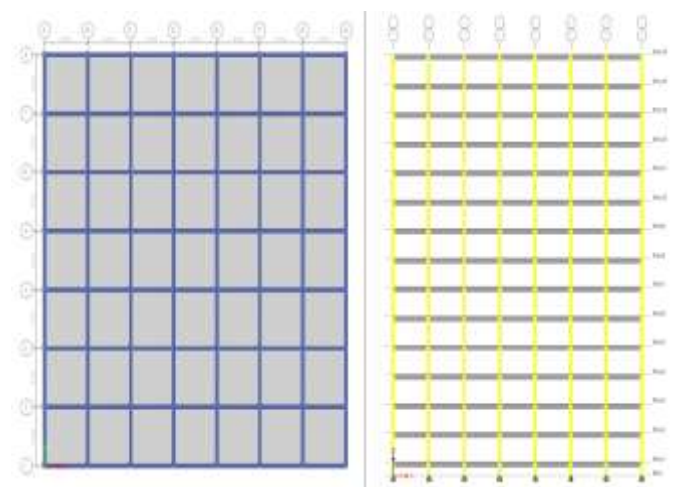


Figure: 4 Plan and elevation view of conventional slab structure

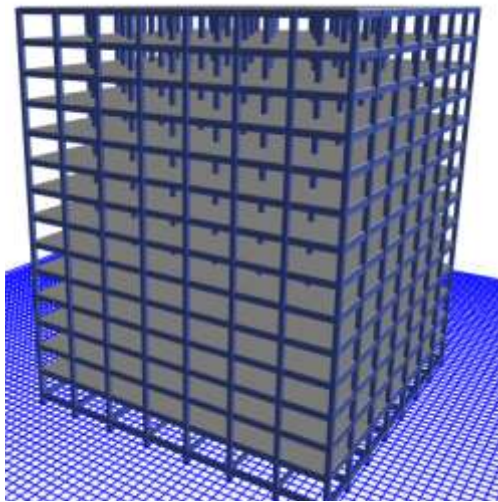


Figure: 5 Plan and elevation view of conventional slab Structure.

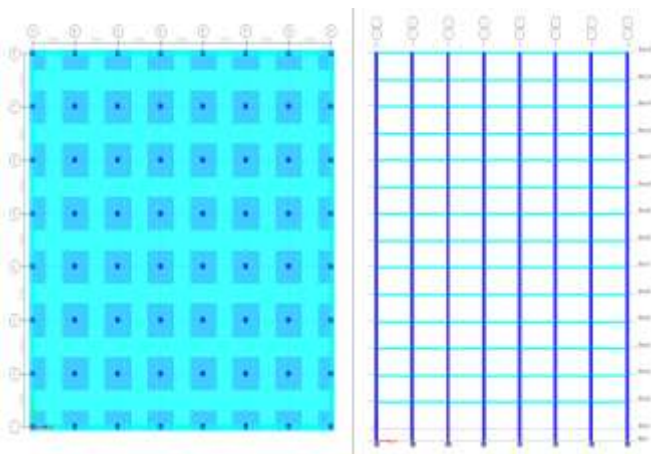


Figure: 6 Flat slab structure without shear wall.

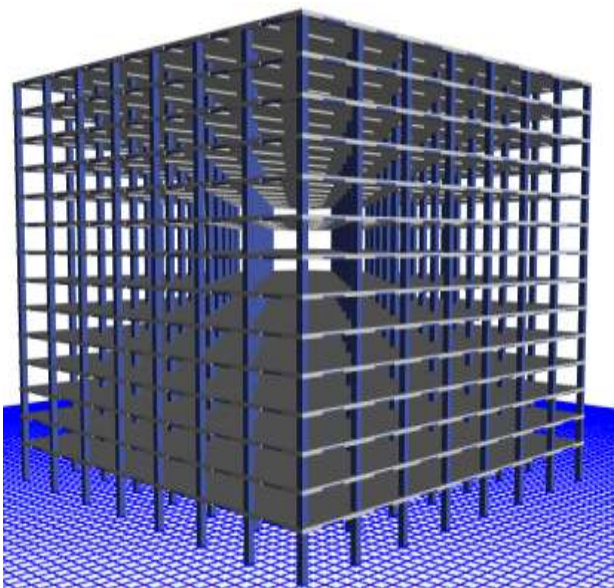


Figure: 7 Rendered view of flat slab structure

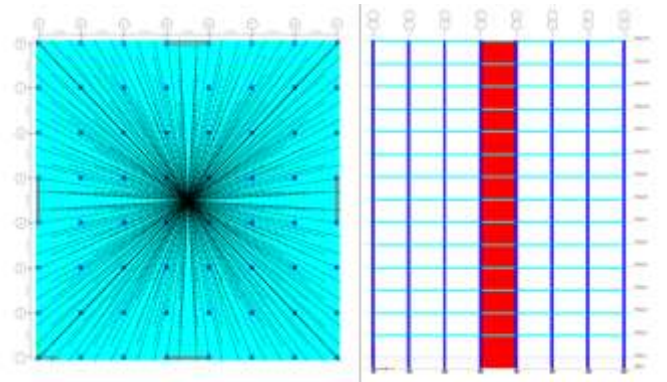


Figure: 8 Flat slab structure with shear wall.

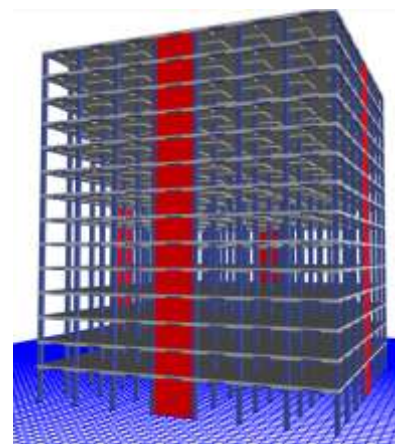


Figure: 9 Rendered view of flat slab with shear wall structure.

7. RESULTS AND DISCUSSION

DISPLACEMENT: It is total displacement of ith storey with respect to ground and there is maximum permissible limit prescribed in IS codes for buildings.

Table; 4 Storey Displacement for ZONE-II

STOREY DISPLACEMENT ALONG EQX			
STOREY	CS WO SW	FS WO SW	FS W SW
15	9.936	98.48	32.542
14	9.716	91.454	29.248
13	9.363	83.869	25.973
12	8.888	75.804	22.743
11	8.309	67.347	19.583
10	7.644	58.595	16.526
9	6.91	49.656	13.605
8	6.121	40.662	10.855
7	5.29	31.776	8.317
6	4.429	23.215	6.031
5	3.55	15.283	4.041
4	2.66	8.421	2.39
3	1.768	3.284	1.128
2	0.893	0.847	0.387
1	0.134	0.182	0.088
0	0	0	0



Chart-1 Plot storey v/s displacement for structure along EQX.

Table; 5 Story Displacement for ZONE-III

STOREY DISPLACEMENT ALONG EQX			
STOREY	CS WO SW	FS WO SW	FS W SW
15	19.124	15.01	52.067
14	18.003	14.254	46.796
13	16.802	13.434	41.557
12	15.534	12.558	36.388
11	14.211	11.635	31.333
10	12.844	10.673	26.442
9	11.444	9.68	21.768
8	10.02	8.661	17.369
7	8.58	7.621	13.307
6	7.132	6.567	9.649
5	5.684	5.502	6.465
4	4.241	4.43	3.824
3	2.81	3.35	1.805
2	1.416	2.226	0.619
1	0.213	0.544	0.141
0	0	0	0

12	15.534	12.558	36.388
11	14.211	11.635	31.333
10	12.844	10.673	26.442
9	11.444	9.68	21.768
8	10.02	8.661	17.369
7	8.58	7.621	13.307
6	7.132	6.567	9.649
5	5.684	5.502	6.465
4	4.241	4.43	3.824
3	2.81	3.35	1.805
2	1.416	2.226	0.619
1	0.213	0.544	0.141
0	0	0	0



Chart-3 Plot storey vs displacement for structure along EQX.

Table; 7 Story Displacement for ZONE-V

STOREY DISPLACEMENT ALONG EQX			
STOREY	CS WO SW	FS WO SW	FS W SW
15	43.03	354.528	117.15
14	40.506	329.235	105.291
13	37.804	301.927	93.504
12	34.951	272.894	81.873
11	31.975	242.449	70.5
10	28.9	210.941	59.494
9	25.749	178.762	48.977
8	22.545	146.383	39.079
7	19.305	114.393	29.941
6	16.048	83.572	21.71
5	12.789	55.018	14.547
4	9.542	30.316	8.603
3	6.324	11.823	4.062
2	3.186	3.048	1.392
1	0.478	0.657	0.317
0	0	0	0

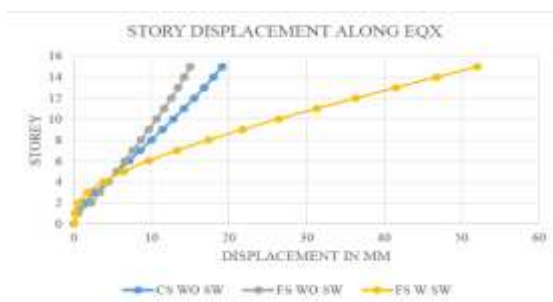


Chart-2 Plot storey v/s displacement for structure along EQX.

Table; 6 Story Displacement for ZONE-IV

STOREY DISPLACEMENT ALONG EQX			
STOREY	CS WO SW	FS WO SW	FS W SW
15	19.124	15.01	52.067
14	18.003	14.254	46.796
13	16.802	13.434	41.557

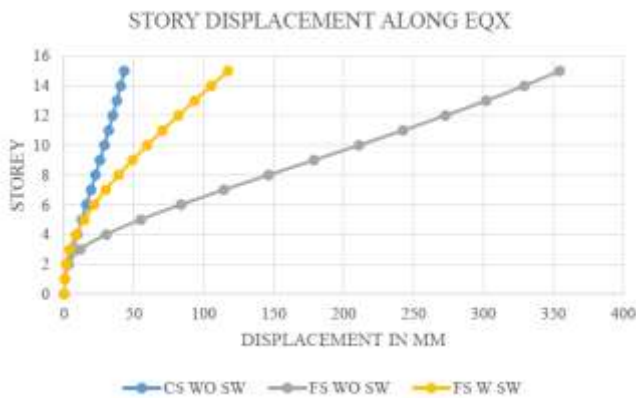


Chart-4 Plot storey vs displacement for structure along EQX.

Story Drift

It is defined as ratio of displacement of two consecutive floor to height of that floor. It is very important term used for research purpose in earthquake engineering.

Table; 8 Story Drift for Zone-II

STOREY DRIFT ALONG EQX			
STOREY	CS WO SW	FS WO SW	FS W SW
15	7.30E-05	0.002342	0.001098
14	0.000118	0.002528	0.001091
13	0.000158	0.002688	0.001077
12	0.000193	0.002819	0.001053
11	0.000222	0.002917	0.001019
10	0.000245	0.002979	0.000974
9	0.000263	0.002998	0.000917
8	0.000277	0.002963	0.000846
7	0.000287	0.002854	0.000762
6	0.000293	0.002645	0.000664
5	0.000297	0.002289	0.000551
4	0.000297	0.001714	0.000429
3	0.000292	0.000812	0.000247
2	0.000253	0.000222	0.000104
1	8.90E-05	0.000122	5.90E-05
0	0	0	0

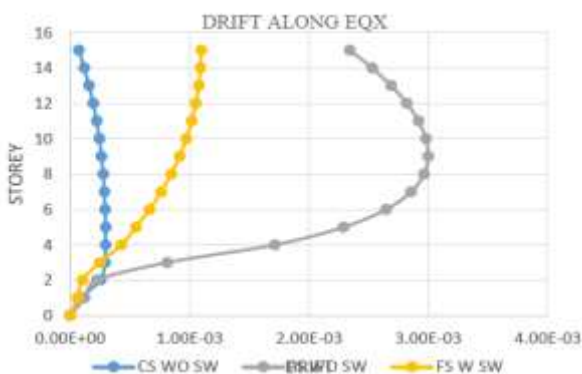


Chart-5 Plot storey vs drift for structures along EQX.

Table; 9 Story Drift for Zone-V

STOREY DRIFT ALONG EQX			
STOREY	CS WO SW	FS WO SW	FS W SW
15	0.000841	0.008431	0.003953
14	0.000901	0.009103	0.003929
13	0.000951	0.009678	0.003877
12	0.000992	0.010148	0.003791
11	0.001025	0.010503	0.003669
10	0.00105	0.010726	0.003506
9	0.001068	0.010794	0.003299
8	0.00108	0.010665	0.003047
7	0.001086	0.010275	0.002744
6	0.001086	0.009521	0.002389
5	0.001082	0.008239	0.001984
4	0.001073	0.006171	0.001543
3	0.001046	0.002925	0.00089
2	0.000903	0.000798	0.000374
1	0.000319	0.000438	0.000211
0	0	0	0

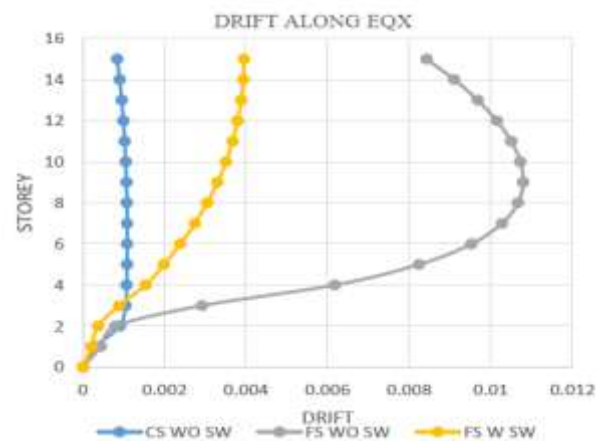


Chart-6 Plot storey vs drift for structures along EQX.

STOREY STIFFNESS

Story Stiffness is the ratio of story force to average drift experienced by each storey. And also Conventional slab and Flat slab is compared for this parameter. If structures are stiff then it's suitable for long period of sites

Table; 10 Storey stiffness for ZONE-II

STOREY STIFFNESS IN KN/m			
STOREY	CS WO SW	FS WO SW	FS W SW
15	1149667.302	64872.816	143068.638
14	1164747.97	65227.945	156228.648
13	1177445.783	65499.248	169044.643
12	1188669.727	65819.097	182146.876
11	1199051.157	66306.619	196253.48
10	1209058.143	67106.466	212275.16
9	1219060.686	68436.216	231473.246
8	1229370.225	70658.961	255757.706
7	1240266.267	74466.805	288323.856
6	1252036.24	81318.023	335061.316

5	1265141.553	94916.052	407702.663
4	1281725.181	128199.71	531030.038
3	1317120.084	283184.527	970218.788
2	1527475.333	980182.376	2203306.44
1	8658652.253	3580337.215	8230752.54
0	0	0	0

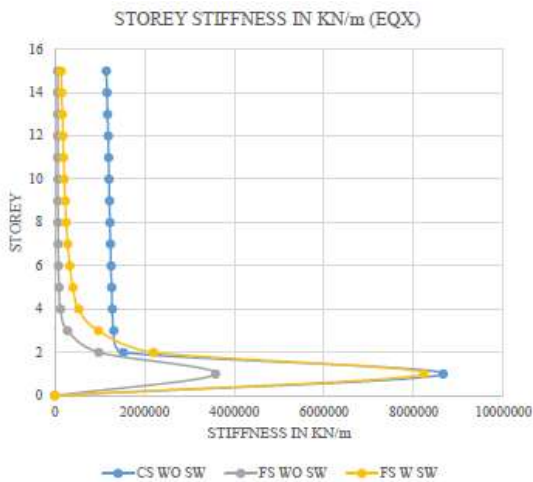


Chart-7 Plot storey vs storey stiffness for structures along EQX.

Table; 11 Storey stiffness for ZONE-V

STOREY STIFFNESS IN KN/m			
STOREY	CS WO SW	FS WO SW	FS W SW
15	1149667.302	64872.816	143068.64
14	1164747.97	65227.945	156228.65
13	1177445.783	65499.248	169044.64
12	1188669.727	65819.097	182146.88
11	1199051.157	66306.619	196253.48
10	1209058.143	67106.466	212275.16
9	1219060.686	68436.216	231473.25
8	1229370.225	70658.961	255757.71
7	1240266.267	74466.805	288323.86
6	1252036.24	81318.023	335061.32
5	1265141.553	94916.052	407702.66
4	1281725.181	128199.71	531030.04
3	1317120.084	283184.527	970218.79
2	1527475.333	980182.376	2203306.5
1	8658652.253	3580337.215	8230752.5
0	0	0	0

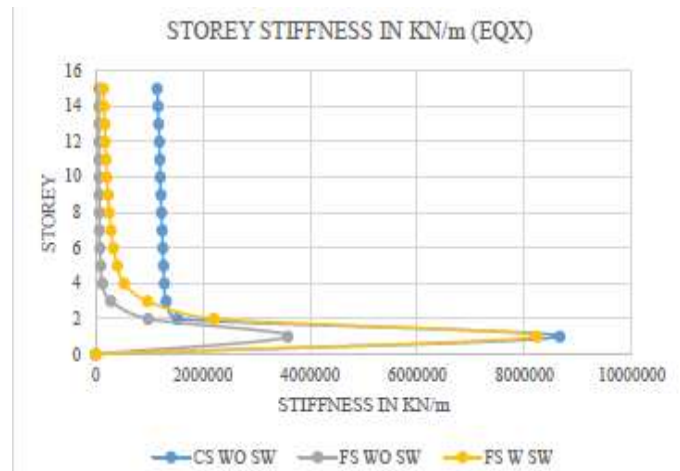


Chart-8 Plot storey vs storey stiffness for structures along EQX.

NATURAL TIME PERIOD

A time period is the time needed for one complete cycle of vibration to pass a given point.

Table; 12 Time period for ZONE-II

TIME PERIOD IN (Sec)			
STOREY	CS WO SW	FS WO SW	FS W SW
1	2.301	8.589	5.042
2	2.301	8.589	5.042
3	2.063	5.318	3.048
4	0.758	2.626	1.084
5	0.758	2.626	1.084
6	0.684	1.703	0.666
7	0.441	1.369	0.439
8	0.441	1.369	0.439
9	0.404	0.952	0.272
10	0.311	0.838	0.251
11	0.311	0.838	0.251
12	0.285	0.623	0.173

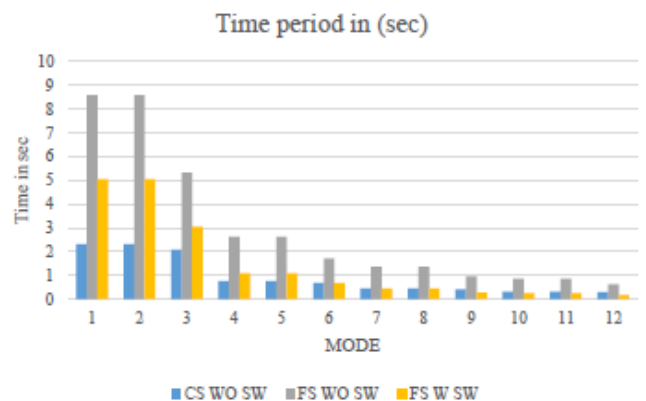


Chart-9 Plot Time vs mode for structures with seismic.

8. CONCLUSIONS

- Displacement of industrial and commercial structure constructed using flat slab system is more than the conventional slab system. Here we can say that flat slab with shear wall gives better displacement resisting.
- With the increase in height of structure displacement is also goes on increasing.
- Story shear of Flat slab building is less than conventional slab building in Y-direction.
- Story shear is maximum at base level and it decreases as height of structure increases.
- Base shear of flat slab building is less than the base shear in conventional slab building in both X and Y directions
- It is seen that story drift is maximum for the conventional slab compared to flat slab and very less for the flat slab with shear wall.
- Story stiffness of conventional slab building is stiffer than Flat slab building. As the story no decreases stiffness goes on increasing.

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