

# AN EXPERIMENTAL INVESTIGATION ON PARTIAL REPLACEMENT OF CRUMB RUBBER FOR SAND AND SILICA FUME FOR CEMENT IN CONCRETE

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## ABSTRACT

A lot of rubber is produced worldwide. For example, 110 million tons rubber is produced annually only in India. It is not possible to discharge the rubbers in the environment because they decompose very slowly and cause lots of pollution. So, it is necessary to have a relevant use of these wastages. These waste materials can be used to improve some mechanical properties of concrete, such as more energy absorption, better ductility, and better crack resistance. In this paper, the 7- day and 28- day compressive strength of concretes containing crumb rubber and silica fume is investigated. The purpose of this experimental investigation is to study the behaviour of strength from Crumb rubber concrete (CRC). In this investigation CRC was manufactured by usual ingredients such as cement, fine aggregate, coarse aggregate, water and mineral admixtures such as silica fume (SF) at various replacement level and certain percentage of crumb rubber. The constant water cement ratio is 0.50. The concrete used in this investigation was proportioned to a target mean strength of 20MPa. Twelve mixes are cast with 5%, 10%, 15%, replacement of cement with silica fume and replacement of fine aggregate with crumb rubber 5%, to study the mechanical properties such as compressive strength, split tensile strength, flexural strength. This paper presents the results on the structural behaviour of CRSF3 RC beam and its comparison with Ordinary RC beam. The concrete is of  $M_{20}$  and the reinforced concrete beams of size 150 mm x 150 mm x 1000 mm were prepared to study the structural behaviour.

**Keywords:** Crumb Rubber & Silica Fume

## 1. INTRODUCTION

### 1.1 GENERAL

Waste materials resulting from various physical and chemical processes are the most important challenge in the industrial and

developing countries. Extensive investigations on wastage recycling are being implemented to minimize the environmental damages. One of the non-recyclable materials enters the environment is automotive used tires. Large quantities of scrap tires are generated each year globally. This is dangerous not only due to potential environmental threat, but also from fire hazards. Over the years, disposal of tires has become one of the serious problems in environments. Land filling is becoming unacceptable because of the rapid depletion of available sites for waste disposal. Investigations show that used tires are composed of materials, which do not decompose under environmental conditions and cause serious contaminations. Burning is a choice for their decomposition; however, the gases exhausted from the tire burning results in harmful pollutions. In order to prevent the environmental problem from growing, recycling tire is an innovative idea or way in this case. Recycling tire is the processes of recycling vehicles tires that are no longer suitable for use on vehicles due to wear or irreparable damage. Based on examinations, another way is using the tires in concrete. This results in the improvement of such mechanical and as energy adsorption, ductility, and resistance to cracking. However, this may cause a decrease in compressive strength of the concrete. The cracker mill process tears apart or reduces the size of tire rubber by passing the material between rotating corrugated steel drums. By process an irregularly shaped torn particles having large surface area are produced and this particles are commonly known as crumb rubber. Mixing of crumb rubber particles with Portland cement concrete does not involve high temperature mixing hence final bond between the cement strength of the matrix.

Portland cement concrete with addition of crumb rubber becomes a heterogeneous mixture due to different specific gravities of the ingredients. The past research shows that although the crumb

rubber Portland cement concrete causes the reduction in strength, it improves certain durability aspects such as freeze thaw resistance, sound absorption, and damping properties and reduces water absorption.

Silicon metal and alloys are produced in electric furnaces. The raw materials are quartz, coal, and woodchips. The smoke that results from furnace operation is collected and sold as silica fume, rather than being land filled. In recent years, using silica fume in concrete in order to increase its strength has attracted much attention. Filling capability and pozzolanic property of silica fume results in filling the capillary gel pores with this material and its compounds with calcium hydroxide. This phenomenon increases the concrete strength significantly. The reaction of silica fume in concrete depends on the amount of this material. One of the main features of silica fume, which improves the properties of fresh and hardened concrete, is its fine particles. Most of the silica fume particles are in the range of 0.01- 0.3 micrometre and their mean particle size ranges between 0.1- 0.2 micrometre. It is apparent that silica grains, which are 100 times smaller than the cement particles, fill the free spaces between the cement and increase the concrete strength. Silica fume is available from suppliers of concrete admixtures and, when specified, is simply added during concrete production. Placing, finishing, and curing silica-fume concrete require special attention.

## 1.2 CRUMB RUBBER

Crumb rubber is a term usually applied to recycled rubber from automotive and truck scrap tires. During the recycling process steel and fluff is removed leaving tire rubber with a granular consistency. Continued processing with a granulator and/or cracker mill, possibly with the aid of cryogenics or mechanical means, reduces the size of the particles further. The particles are sized and classified based on various criteria including colour (black only or black and white). The granulate is sized by passing through a screen, the size based on a dimension (1/4") or mesh (holes per inch: 10, 20, etc.). Mesh refers to material that has been sized by passing through a screen with a given number of holes per inch. For example, 10 mesh crumb rubber has passed through a screen with 10 holes per inch resulting in rubber granulate that is slightly less than 1/10 of an inch. The exact

size will depend on the size of wire used in the screen.

*Fig 1 Crumb rubber*



## 1.3 SILICA FUMES

Silica fumes also known as micro silica is a fine-grain, thin, and very high surface area silica. It is sometimes confused with fumed silica (also known as pyrogenic silica) and colloidal silica. These materials have different derivations, technical characteristics and application. Silica fumes is a by-product silicon metal or ferrosilicon alloys. One of the most beneficial uses for silica fumes is in concrete. Because of its chemical and physical properties, it is a very reactive pozzolana. Concrete containing silica fumes can have high strength and can be very durable. Silica fume is available from suppliers of concrete admixtures and, when specified is simply added during concrete production. Placing, finishing, and curing silica-fumes concrete require special attention on the part of the concrete contractor. A silica fume consists primarily of amorphous (non-crystalline) silicon dioxide (SiO<sub>2</sub>). The individual particles are extremely small, approximately 1/1000th the size of an average cement particle. Because of its fine particles, large surface area, and the high SiO<sub>2</sub> content, silica fumes is a very reactive pozzolana when used in concrete. Silica fume for use in concrete is available in wet and dry forms. It is usually added during concrete production at a concrete plant as shown in the photo.

*Fig.2 Silica fume*



### 1.3.1 SILICA FUMES WORK IN CONCRETE

In cementitious compounds, silica fumes works on two levels, the first one described here is a chemical reaction called the pozzolanic reaction. The hydration (Mixing with water) of Portland cement produces many compounds including calcium silica hydrates (CSH) and calcium hydroxide (CH). The CSH gel is known to be the source of strength in concrete. When silica fumes is added to fresh concrete it chemically reacts with the CH to produces additional CSH. The benefit of this reaction is twofold, increased compressive strength and chemical resistance. The bond between the concrete paste and the coarse aggregate, in the crucial interfacial zone is greatly increased, resulting in compressive strengths that can exceed 15000 psi. The additional CSH produced by silica fume is more resistant to attack from aggressive chemicals than the weaker CH. The second function silica fume performs in cementitious compound is a physical one. Because silica fumes is 100 to 150 times than a cement particle it can fill the voids created by free water in the matrix. This function, called particle, packing, refines the microstructure of concrete, creating a much denser pore structure. Impermeability is dramatically increased, because silica fume reduces the number and size of capillaries that would normally enable contaminants to infiltrate the concrete. Thus silica fumes modified concrete is not only stronger, it lasts longer, because it's more resistant to aggressive environments. As filler and pozzolana silica fume's dual action in cementitious compounds are evident throughout the entire hydration process.

### 1.4. SCOPE

Now a day due to the rapid industrial growth, waste material management is a challenging field. It possesses lot of environmental impact. Due to the rapid growth in construction field, construction material scarcities will arise. So we need to find some alternate material for construction. Crumb rubber is a waste material from the scrap tyre. By using this as fine aggregate we can prevent the natural aggregate depletion. This avoids so much of environmental problems.

### 1.5. OBJECTIVES

The main object of investigation is to study the strength behaviours of crumb rubber concrete with economical basis. Partially replacement of cement

by using silica fumes up to 15%. Partially replacement of sand by using crumb rubber up to 5%. To investigate the use of crumb rubber in conventional concrete. Find The Compressive Strength of concrete cubes with 5% to 15% replacement of fine aggregate by crumb rubber with silica fume. Find The Split Tensile strength of concrete cylinder. Find The Flexural strength of concrete beam. Comparisons of all the above results with ordinary concrete.

## 2. REVIEW OF LITERATURE

### A. Khan., S. Danish., S. Arif., S. Ramzan., M. Mushtaq (2013)

This paper, through experimental study and literary sources investigates the utilization of rubber waste in developing Green Concrete (GC). The natural aggregate (sand) of Conventional Concrete (CC) is replaced as 10%, 20% and 30% with coarse and fine rubber aggregate. The samples were tested in laboratory after a specific time on various aspects including compression strength and results were compared with each other and also with Conventional Concrete Mix (CCM). The paper concludes that structural and non-structural rubberized concrete can be developed by using specific quantity of rubber waste in placement of fine and coarse aggregates in conventional concrete. The key objective of this research is to find out an efficient solution for utilizing rubber waste for better environment and also to provide initiative for concerned government organization for framing effective legislation for the use of rubberized concrete in building and construction industries. It reveals from compressive strength's tests of rubberized concrete (RC) that it can be produced for various use in building and construction Industries. RC will not only save the natural ingredients of concrete resulting environmental sustainability but recycling of rubber waste will also contribute towards better environment. The replacement of 10 % of fine aggregate (sand) of Conventional Concrete (CC) with Fine Crumb Rubber (FCR) is useful for producing Structural Concrete. Therefore use of CCR concrete should be encouraged for use so that maximum consumption of waste rubber could be achieved.

### K. C. Panda, P. S. Parhi and T. Jena (2012)

In this study an attempt has been made to identify the various properties necessary for the

design of concrete mix with the coarse tyre rubber chips as aggregate in a systematic manner. In the present experimental investigation, the M20 grade concrete has been chosen as the reference concrete specimen. Scrap tyre rubber chips, has been used as coarse aggregate with the replacement of conventional coarse aggregate. Slump value is decreased as the percentage of replacement of scrap tyre rubber increased. So decrease in workability. The compressive strength is decreased as the percentage of replacement increased, but rubber concrete developed slightly higher compressive strength than those of without rubber concrete. The split tensile strength is increased with decreased percentage of scrap tyre rubber. Decrease in compressive strength, split tensile strength and flexural strength of the specimen. Lack of proper bonding between rubber and cement paste matrix.

**G.SenthilKumaran,  
M.Lakshmipathy (2008)**

**NurdinMushule,**

This study reviews the feasibility of using waste tires in the form of chips and fibers with different sizes in concrete to improve the strength as well as protecting the environment. Also it reviews the potential application in the field by exploiting its unique characteristics and properties. In this study, we outline the use of rubberized concrete in structural and non-structural members and show how it is suitable for the concrete, its uses, barriers and benefits and way to future study. A research is underway using the grade of cement 53, to improve the strength, fine sand and coarse aggregate of a combination of 10mm and 20mm. The waste tyre rubber shall be used in the form of chips and fibers by partially replacing the coarse aggregate by 0, 5, 10, 20 and 25%. Recycling technology for concrete has significantly developed in the recent years, making the material sufficiently recyclable. It is evident that from the above discussion, the reduction of compressive and tensile strength can be increased by adding some super plasticizers and industrial wastes as partial replacement of cement will definitely increase the strength of waste tyre rubber modified concrete. Many studies reveal that there will be increase in strength enhancements as well as environmental advantages. The future NGC using waste tyre rubber could provide one of the environmental friendly and economically viable products. Though problems remain regarding the cost of production and awareness among the society the wastes can be

converted into a valuable product But further research is needed to increase performance against fire.

**El-Gammal, A., A. K. Abdel-Gawad, Y. El-Sherbini,  
and A. Shalaby (2010)**

In this paper the density and compressive strength of concrete utilizing waster tire rubber has been investigated. Recycled waste tire rubber has been used in this study to replace the fine and coarse aggregate by weight using different percentages. The results of this paper shows that although, there was a significant reduction in the compressive strength of concrete utilizing waste tire rubber than normal concrete, concrete utilizing waste tire rubber demonstrated a ductile, plastic failure rather than brittle failure. A total of 4 main mixtures were cast. One control mixture and three concrete mixtures. The control mixture was designed to have a water cement ratio of 0.35 with cement content of 350 kg/m<sup>3</sup>. To develop the rubberized concrete mixtures, tire rubber was used to replace the aggregate by weight. In the first rubberized concrete mixture, the chipped rubber totally replaced the coarse aggregate in the mixture. While, in the other two concrete mixtures, the tire rubber replaced the fine aggregate by 100% and 50% of fine aggregate weight. Concrete casted using chipped rubber as a full replacement to coarse aggregate shows a significant reduction in the concrete strength compared to the control specimen. However, significant ductility was observed before failure of the specimens. Concrete casted using chipped rubber as a full replacement to coarse aggregate shows a significant reduction in the density of concrete compared to the control specimens. Concrete casted using crumb rubber as a full replacement to sand shows a significant reduction in the concrete strength compared to the control specimen. However, significant ductility was observed before failure of the specimens. Concrete casted using crumb rubber as a full replacement to sand shows a significant increase in the concrete strength compared to the concrete casted using chipped rubber as a replacement to coarse aggregate. There was no significant increase in the concrete compressive strength and the concrete density when different percentage of crumb rubber, as a replacement to sand, was used in the concrete mix. It is recommended to test concrete with different percentage of crumb rubber ranging between (10% up to 25%) to study its effect on the concrete strength.. It is recommended

to use concrete in the production of curbs, roads, concrete blocks, and non-bearing concrete wall.

### **T. SenthilVadivel& R. Thenmozhi (2012)**

In this Study, our present study aims to investigate the optimal use of waste tyre rubber crumbs as fine aggregate in concrete composite. A total of 90 cubes, cylinders and beam specimens were cast with the replacement of fine aggregate by shredded rubber crumbs with the proportion of 2, 4, 6, 8, and 10% by weight and compared with 18 conventional specimens. Fresh and hardened properties of concrete such as workability, compressive strength, tensile strength and flexural strength were identified and finally it is recommended that 6% replacement of waste tyre rubber aggregate with fine aggregate will gives optimal and safest replacement in concrete composites. Compressive strength decreases when the percentage of replacement of shredded fine rubber crumbs increases. Split tensile strength decreases at the maximum of 25% when rubber crumbs replaces up to 10% in fine aggregate. Flexural strength of concrete increases when rubber crumbs increases up to 6%. It is identified that the grade of concrete plays the major role in the ductility performance of rubber replaced Concrete. Slump test results show no change in workability in all the percentage of replacement of rubber crumbs. Hence no effect in consistency during rubber replaced concrete. 6% replacement of waste tyre rubber proves exceptionally well in compression, tensile and flexural strength and follow the curvature of the conventional specimens all the tests in both the grades. Hence it is recommended that 6% replacement of waste tyre rubber aggregate with fine aggregate will gives optimal and safest replacement in concrete composites. Further it is suggested to use this concrete composite for lintel beams, floor slabs, and ribs where load carrying capacity not governing the design.

## **3. METHODOLOGY**

### **3.1 MATERIAL USED**

Various materials used for the experimental study are the following:

### **3.2 CEMENT**

Cement is a material that has cohesive and adhesive properties in the presence of water. Such cement is called hydraulic cements. These consist primarily of silicates and aluminates of lime obtained from limestone and clay. There are different types of cement; out of that I have used one type.

#### **3.2.1 ORDINARY PORTLAND CEMENT**

Ordinary Portland cement (OPC) is the basic Portland cement and is best suited for use in general concrete construction. It is of three types, 33 grades, 43grade, 53 grade. One of the important benefits is the faster rate of development of strength. Cement is a fine, grey powder. It is mixed with water and materials such as sand, gravel, and crushed stone to make concrete. The cement and water form a paste that binds the other materials together as the concrete hardens. The basic composition of cement is provided in table. In the present work 43 grade cement was used for testing.

### **3.3 FINE AGGREGATE**

Locally available free of debris and nearly riverbed sand is used as fine aggregate. Among various characteristics, the most important one is its grading coarse sand may be preferred as fine sand increases the water demand of concrete and very fine sand may not be essential in as it usually has larger content of thin particles in the form of cement. The sand particles should also pack to give minimum void ratio, higher voids content leads to requirement of more mixing water.

### **3. 4 WATER**

Water is an important ingredient of concrete as it actively participates in the chemical reactions with cement. The strength of cement concrete comes mainly from the binding action of the hydration of cement get the requirement of water should be reduced to that required chemical reaction of unhydrated cement as the excess water would end up in only formation undesirable voids or capillaries in the hardened cement paste in concrete. It is important to have the compatibility between the given cement and the chemical material admixtures along with the water used for mixing. It is generally stated in the concrete codes and also in the literature that the water fit for drinking is fit for making concrete. This may not be

true always. For example some water containing a small amount of sugar would be suitable for drinking, but they are good for cement concrete, as the sugar would adversely affect the hydration process. The limits of the content of water have to be determined from the following consideration. High content of cement is susceptible to a rapid loss of workability on Account of higher amount of heat of hydration generated. Therefore attention is required to see that the initial hydration rate of cement should not be significantly affected. The salt in water would not interface with the development of strength of later ages. Apart from the strength consideration, the durability characteristics such as porosity, degree of resistance to diffusion of CO<sub>2</sub>, CaSO<sub>4</sub>, moisture, air oxygen, etc. should also be investigated after specified curing period.

### 3.5 MIX DESIGN

#### 3.5.1 DEFINITION

Mix design is the process of selecting suitable ingredient if concrete and determines their relative proportion with the object of certain minimum strength and durability as economically as possible.

#### 3.5.2 DESIGN OF M20 GRADE CONCRETE

##### Design parameters

Characteristic strength: 20 N/mm<sup>2</sup>

Compaction factor: 0.85

Degree of quality control: Good

Type of exposure: severe

##### Data on material

Cement used: Grade 53 conforming to IS: 12269-1987

Specific gravity of cement: 3.15

Sand: Conforming to zone II

Specific gravity of fine aggregate: 2.62

Bulk density of fine aggregate: 1.698

Fineness of modulus: 2.71

Specific gravity of coarse aggregate: 2.72

Bulk density of coarse aggregate: 1.82

Fineness of modulus: 2.59

Compaction factor: 0.80

##### Bureau of Indian Standards method

(a). Target mean strength for specified characteristic cube strength is

$$F_{ck} = f_{ck} + t_s$$

$$= 20 + 1.65 \times 4$$

$$= 26.56 \text{ N/mm}^2$$

(b). Selection of water cement ratio required for the target mean strength of 26.56 N/mm<sup>2</sup> is maximum free W/C ratio is 0.5 from IS 456-2000

(c). Selection of water and cement content

20mm maximum size aggregate sand containing to grading zone II. Water content per cubic meter of concrete 186 kg and sand content as percentage of total aggregate by absolute volume 35%

Correction for decrease in water cement ratio:

Total aggregate by absolute volume is 35-3 = 32%

(d). Determination of cement content

$$\text{W/C ratio} = 0.50$$

$$\text{Water} = 186 \text{ lit}$$

$$\text{Cement} = 186 / 0.50 = 372 \text{ Kg/m}^3$$

(e). Determination of coarse and fine aggregate.

The amount of entrapped air is 2% and crumb rubber is 5%

$$\text{Absolute volume of concrete, } V = 1 - (0.02 + 0.10) = 0.93$$

Determination of fine aggregate:

$$V = \{W + C/Se + (1/P \times fa/Sfa)\} \times 1/1000$$

$$0.93 = \{186 + 372/3.15 + (1/0.32 \times fa/2.62)\} \times 1/1000$$

$$FA = 525 \text{ kg/m}^3 \text{ (Similarly for coarse aggregate)}$$

$$V = \{W + C/Se + [1 / (1-P) \times (Ca/Sca)]\} \times 1/1000$$

$$0.93 = \{186 + 372/3.15 + (1/0.68 \times Ca/2.67)\} \times 1/1000$$

$$\text{Course aggregate, } CA = 1137 \text{ Kg/m}^3$$

The mix proportion per cubic meter

$$\text{Water} = 186 \text{ lit}$$

$$\text{Cement} = 372 \text{ Kg}$$

$$\text{Fine aggregate} = 525 \text{ Kg}$$

$$\text{Course aggregate} = 1137 \text{ Kg}$$

### 3.5.3 MIX PROPORTION DETAILS

Table :1 Mix proportion for a cubic meter of concrete

Mix	Cement (Kg/m <sup>3</sup> )	Crumb rubber (Kg/m <sup>3</sup> )	Silica fume (Kg/m <sup>3</sup> )	FA (Kg/m <sup>3</sup> )	CA (Kg/m <sup>3</sup> )	Water (lit/m <sup>3</sup> )
M20	372	0	0	537	1244	186
CRSF 1	372	26.25	0	499	1137	186
CRSF 2	354	26.25	18.6	499	1137	186
CRSF 3	335	26.25	37.2	499	1137	186
CRSF 4	316	26.25	55.8	499	1137	186

Where

CRSF 1 = Crumb rubber (CR) 5% replacement of fine aggregate, Silica fume (SF) 0% replacement of cement.

CRSF 2 = Crumb rubber (CR) 5% replacement of fine aggregate, Silica fume (SF) 5% replacement of cement.

CRSF 3 = Crumb rubber (CR) 5% replacement of fine aggregate, Silica fume (SF) 10% replacement of cement.

CRSF 4 = Crumb rubber (CR) 5% replacement of fine aggregate, Silica fume (SF) 15% replacement of cement.

### 3.6 DETAILS OF FLEXURAL MEMBERS CASTED

CRSF5 Beams of size 150 x150x1000mm were casted and they are compared with ordinary concrete.

Table: 2 Casted Beam Details (150x150x1000mm)

Beam details	Number of specimens
M <sub>20</sub>	1
CRSF3	1

#### 3.6.1 Reinforcement Details

Main reinforcement -4 bars of 8 mm diameter

Shear reinforcement - 8 mm diameter bars @150 mm c/c

#### 3.6.2 Experimental Setup

All the two beams were tested at the age of 28 days in a two point loading conditions, two point loads were applied to the beams by hydraulic testing machine up to the failure. The deflection was measured at the mid span by using dial gauges. The loads were applied in small increments and at every increment of loading, the deflection was recorded.

### 4. RESULTS AND DISCUSSIONS

#### 4.1 COMPRESSIVE TEST OF CONCRETE CUBE

The compression test is carried out with cube specimen to find out the compressive strengths of conventional and rubber replaced concretes using compression testing machine and the results are tabulated

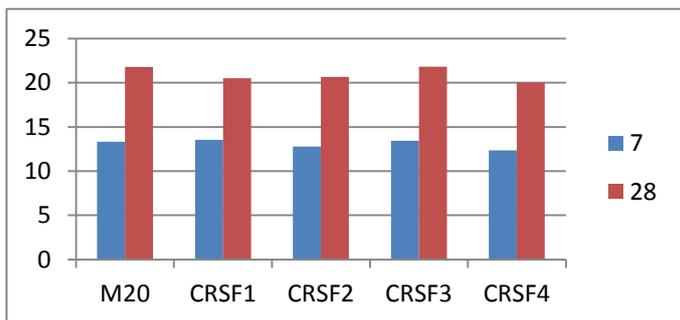
Fig :3 Experimental Setup for Testing of Cube Specimens



	7 days specimen		28 days Specimen	
	Collap se load (KN)	Comp. strength(N/mm <sup>2</sup> )	Collap se load (KN)	Comp. strength(N/mm <sup>2</sup> )
M20	300	13.33	490	21.77
CRS F1	350	13.32	595	20.66
CRS F2	310	12.77	510	20.66
CRS F3	280	13.44	450	21.81
CRS F4	300	12.33	439	20.01

Table: 3 Compressive strength results

Graph: 1 Comparing Compressive strength for partial replacement of crumb rubber for sand and silica fume for cement in standard concrete



### 4.3 SPLIT TENSILE STRENGTH

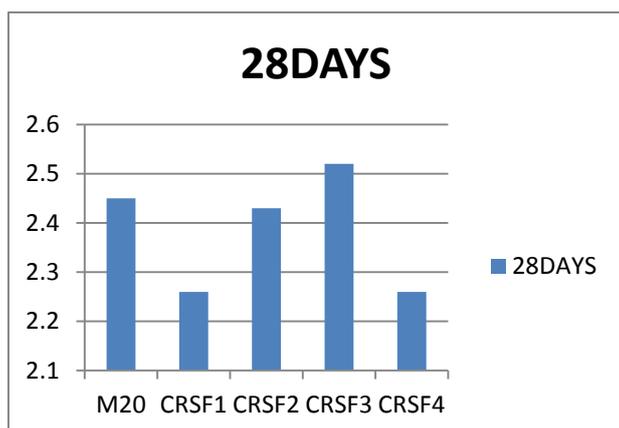
The split tensile test has been carried out and comparative results conventional and rubber replaced concretes are tabulate.

Fig 4 Experimental Setup for Testing of Cylinder Specimens



Table: 4 Tensile strength results

Graph: 2 Comparing split tensile strength partial replacement of crumb rubber for sand and silica fume for cement in standard concrete



### 4.4 EXPERIMENTAL RESULTS FOR ORDINARY RCC BEAM AND CRSF5 BEAM

#### 4.4.1 Load and deflection for ordinary RCC beam

The deflection was measured at one points using the dial gauge, fixed at the mid span. The deflection increased according to load increases. Load and Displacement values for ordinary reinforced concrete beam are shown in table

Table: 5 load and deflection for ordinary RCC beam

S. no.	Load (KN)	Deflection in mm (at centre)
1	2.5	0
2	5	0.34
3	7.5	0.43
4	10	0.94
5	12.5	1.10
6	15	1.85
7	17.5	2.80
8	20	3.75
9	22.5	5.25
10	25	6.80
11	27.5	7.80
12	30	8.60
13	32.5	Ultimate load

Cube	28 days Specimen	
	Collapse load (KN)	Tensile strength(N/mm <sup>2</sup> )
M20	150	2.45
CRSF 1	130	2.26
CRSF 2	135	2.43
CRSF 3	152	2.51
CRSF 4	130	2.26

#### 4.4.2 Load and deflection for CRSF3 RCC beam

The deflection was measured at one points using the dial gauge, fixed at the mid span. The deflection increased according to load increases. Load and Displacement values for CRSF3 reinforced concrete beam.

Table: 6 load and deflection for CRSF3 beam

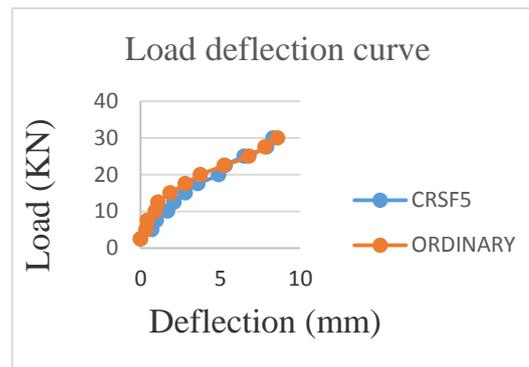
S. no.	Load in (KN)	Deflection in mm (at centre)
1	2.5	0
2	5	0.70
3	7.5	1.00
4	10	1.69
5	12.5	2.10
6	15	2.80
7	17.5	3.60
8	20	4.90
9	22.5	5.30
10	25	6.50
11	27.5	7.90
12	30	8.30
13	32.5	9.10
14	35	Ultimate

#### 4.5 COMPARISON BETWEEN ORDINARY RC BEAM AND CRSF3 BEAM

Table 7 Comparison between Ordinary RC Beam and CRSF3 Beam

Beam ID	Ultimate Load (KN)		Deflection (mm)	
	Ordinary	CRSF3	Ordinary	CRSF3
Results	32.5	35	8.6	11.80

Graph: 3 Load Deflection Curve for Ordinary beam and CRSF



#### 5. CONCLUSIONS

The 7- day and 28- day compressive strength of the specimens increased by addition of silica fume to concrete containing crumb rubber. This happens because of filling capability of silica fume fine particles as well as good adhesion between the rubber and the cement paste.

Based on the above test results concluded the following:

Compressive strength decreases when the percentage of replacement of shredded fine silica fume increases.

Split tensile strength decreases at the maximum of 15% when rubber crumbs replaces up to 5% in fine aggregate.

Flexural strength of concrete increases when rubber crumbs increases up to 5% and silica fume upto 10%.

It is identified that the grade of concrete plays the major role in the performance of rubber replaced concrete

CRSF 3 (10) % replacement of waste tyre rubber proves exceptionally well in compression, tensile and flexural strength and follow the curvature of the conventional specimens all the tests in M<sub>20</sub> grades.

Hence it is recommended that Crumb rubber (CR) 5% replacement of fine aggregate, Silica fume (SF) 10% replacement of cement will gives optimal and safest replacement in concrete composites. Further it is suggested to use this concrete composite for floor slabs, and ribs along with the

lintel beam. Where load carrying capacity not governing the design.

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