# Design and Analysis of Elevated Water Tank 

Tejaswini $\mathbf{R}^{\mathbf{1}}$, Mamatha $\mathrm{A}^{\mathbf{2}}$<br>${ }^{1}$ PG Student, Department of Civil Engineering, East West Institute of Technology, Bengaluru, Karnataka, India. ${ }^{2}$ Associate Professor, Department of Civil Engineering, East West Institute of Technology, Bengaluru, Karnataka, India.


#### Abstract

Water tanks are widely used for storing potable water. Due to lack of water around the world, importance is given more on the water storage project. So water storage is very important as it plays a vital role in everyday life. The recent edition for the design concerning towards liquid retaining structure have been revised. The revised edition incorporated limit state design method. In this method the structure is first designed under limit state of collapse, then checked under serviceability. IS3370:2009 adopts limit state design. The Elevated rectangular RC water tank designed under limit state design method and analysis carried out for the empty tank, full tank condition using linear static analysis(equivalent static method) and linear dynamic analysis(response spectrum method)using ETABS Software. As per the results the area of steel required for the structure increases in limit state method. The limit state method provides more effective reinforcement and it is economical.


Key Words: Elevated rectangular water tank, linear static analysis, linear dynamic analysis.

## 1. INTRODUCTION

Water tanks are widely used for storing potable drinking water. In the current situation there is more emphasis on the water storage project around the world due to lack of water that is spreading. Water plays a very important role in everyday life, so water storage is very important. Not only did they store water and other liquids, they also had products stored in large-scale industries. Liquid retaining tanks have different sorts of supporting structures for example RC shaft, steel frame, RC braced frame and even brick work platform. In general, depending upon the location tanks are classified as tank resting on ground, underground water tank, overhead or elevated water tank. Depending upon their shape water tanks are further classified as rectangular tanks, circular tanks, intze tanks. The design of liquid retaining structures have been revised. The revised edition incorporated limit state design method. The structure is first designed under limit state of collapse and then checked under serviceability. IS 3370:2009 adopts limit state design method with precautions. Limit state design method adopts the criteria for limiting crack width. The principal motivation behind this work is to design elevated rectangular RC water tank using limit state method and analyse the seismic exhibition of the water tank considering, different zones and different soil conditions.

## 2. DESCRIPTION OF MODEL

The model considered here is rectangular elevated water tank of $250 \mathrm{~m}^{3}$ capacity supported on RCC frame, staging of height 10 m and four number of columns. The elevated tank is analysed for various soil condition they are hard, medium and soft soil and various zones were considered they are zone II, III, IV, V. The grade of concrete M30 and grade of steel Fe415 are considered for study. The model is analyzed using linear static analysis and linear dynamic analysis method in ETABS.

### 2.1 Elevated Rectangular tank.

Table -1: Parameters of Elevated Rectangular Tank

| particulars | dimensions |
| :--- | :--- |
| Thickness of wall | 400 mm |
| Free board | 0.3 m |
| Lower slab thickness | 500 mm |
| Bottom ring beam | $500 \times 1000 \mathrm{~mm}$ |
| Size of braces | $300 \times 500 \mathrm{~mm}$ |
| Column size | $500 \times 600 \mathrm{~mm}$ |
| Number of column | 4 |
| Staging height | 10 m |
| Height of tank | 7 m |
| Zone factor | $0.1,0.16,0.24,0.36$ |


| Reduction factor | 2.5 |
| :--- | :--- |
| Importance factor | 1.5 |

## 3. METHODOLOGY

The methodology includes design of elevated rectangular water tank using limit state method as per IS 3370:2009 as well as working stress method and then compared. In this examination the modelling of rectangular RC water tank is done in ETABS software. The manual estimation for seismic analysis has been accomplished using using IS 1893:2014.The analysis is carried in two different methods one is linear static method (Equivalent static method) of analysis and other one is linear dynamic analysis method (Response spectrum analysis). The investigation is accomplished for completely filled water condition and empty water condition for various seismic zones and distinctive soil conditions. The logical examination of a seismic response of raised water tank by response spectrum method by considering the convective mode for an earthquake data also included in the present study.


Fig 1. 3D model of Elevated Rectangular Tank

## 4. RESULTS AND DISCUSSIONS

In this study, elevated rectangular tank have been designed using limit state design method (IS 3370:2009) and working stress method (IS $3370: 1965$ ) and compared. Using ETABS the linearly static investigation and dynamic response spectrum investigation is carried out for empty tank condition and fully tank condition using above mentioned parameters. For each condition separate models have prepared for different soil sites and analysis is done. The results are noted down for base shear, base moment, and displacement at top, axial forces.

### 4.1 Comparative result of Elevated Rectangular RC Water Tank

Table -2: Comparsion of Working Stress Method and Limit State Method

| STRUCTRAL <br> ELEMENT | WORKING <br> METHOD (IS 3370:1965 ) | LIMIT STATE METHOD <br> (IS 3370:2009) |
| :--- | :--- | :--- |
| COLUMN |  |  |
| Area of C/S | $500000 \mathrm{~mm}^{2}$ | $500000 \mathrm{~mm}^{2}$ |
| Area of steel <br> required | $4000 \mathrm{~mm}^{2}$ | $18500 \mathrm{~mm}^{2}$ |
| VERTICAL WALL |  |  |
| wall thickness | 400 mm | 400 mm |
| Area of steel <br> required at support | $2961 \mathrm{~mm}^{2}$ | $3428 \mathrm{~mm}^{2}$ |


| Area of steel <br> required at centre | $2317 \mathrm{~mm}^{2}$ | $3021 \mathrm{~mm}^{2}$ |
| :--- | :--- | :--- |
| BASE SLAB |  |  |
| Thickness of steel | 500 mm | 200 mm |
| Area of <br> required at support | $2314 \mathrm{~mm}^{2}$ |  |
| Area of steel <br> required at centre | $12208 \mathrm{~mm}^{2}$ | $1852 \mathrm{~mm}^{2}$ |

### 4.2 DISPLACEMENT AT TOP STORY

Maximum story displacement values are obtained from ETABS for different soil types in empty tank and full tank condition.

Table -3: Displacement comparison

| DISPLACEMENT COMPARISON (mm) |  |  |
| :--- | :--- | :--- |
| SOIL | EMPTY TANK CONDITION | FULL TANK CONDITION |
| SOFT SOIL | 34.099 | 49.924 |
| MEDIUM <br> SOIL | 45.654 | 68.536 |
| HARD SOIL | 56.605 | 85.703 |



Chart 1: Displacement comparison


Chart 2: Displacement in hard soil Chart 3: Displacement in medium soil


Chart 4 : displacement in soft soil

## Discussion on displacement of the model.

1. The displacement is high in hard soil in full tank conditions compare with the hard soil in empty tank condition.
2. The lateral forces will be high in hard soil compared to other.
3. The graphs shown in chart 2, 3, 4 were obtained from ETABS for hard, medium and soft soil types for empty tank condition.
4. Same procedure were followed for plotting graphs for hard, medium and soft soil types for full tank condition.

### 4.3 BASE SHEAR AND BASE MOMENT

Base shear and base moment values are obtained from ETABS for different soil types in empty tank and full tank condition.
Table -4: Base shear and base moment for soft soil condition

| EMPTY TANK CONDITION |  |  | FULL TANK CONDITION |  |
| :--- | :--- | :--- | :--- | :--- |
| ZONE | BASE SHEAR | BASE MOMENT | BASE SHEAR | BASE MOMENT |
|  | (KN) | $(\mathrm{KN}-\mathrm{m})$ | $(\mathrm{KN})$ | $(\mathrm{KN}-\mathrm{m})$ |
| II | 117.09 | 376.06 | 72.80 | 234.35 |
| III | 185.15 | 599.49 | 115.88 | 375.38 |
| IV | 277.23 | 899.72 | 174.32 | 565.06 |
| V | 416.34 | 1349.59 | 260.48 | 845.08 |



Chart 5: base moment in soft soil condition


Chart 6: base shear in soft soil condition

## Discussion on base shear and base moment of the model.

1. Base shear and base moment for empty tank condition is more compare to full tank condition in all the three soil condition.
2. Because water tank is empty hence no water pressure from inside, only earthquake forces are acting from outer side only.
3. Base shear and base moment for medium soil, hard soil condition were analysed and the results were obtained, procedure for plotting graph were same as of soft soil condition.
4. The base shear and base moment value for medium soil, hard soil was more in empty tank condition compare to full tank condition.

### 4.4 AXIAL FORCES

Axial forces values are obtained from ETABS for different soil types in empty tank and full tank condition.
Table -5: Axial force comparison

| AXIAL FORCES FOR ALL SOIL TYPES 1.5 ( DL+LL ) |  |  |
| :--- | :--- | :---: |
|  | EMPTY TANK | FULL TANK |
| STORY 1 | 28725.75 | 28749.92 |
| STORY 2 | 28237.79 | 28250.95 |
| STORY 3 | 27667.81 | 27791.99 |
| STORY 4 | 19324.007 | 19435.18 |



Chart 7: Axial force comparison

## Discussion on Axial force of the model.

1. Axial forces are increased in the fully tank condition compare with the empty tank conditions.
2. The increased values in full tank condition are very small varying compare to empty tank.

## 5. CONCLUSIONS

1. From results it shows that the area of steel required in limit state method increase when compared to that of working stress method as the allowable stresses in steel were lower.
2. From the above outcomes and discussion, the displacement is high in hard soil in full tank conditions compare with the hard soil in empty tank condition because of in full tank condition lateral forces are more.
3. From the results we conclude that base shear and base moment for empty tank condition is more compare to full tank condition. Because of water tank is empty hence no water pressure from inside, only earthquake forces are acting from outer side only. Hence more base shear and base moment in empty tank condition.
4. Axial forces are increased in the fully tank condition compare with the empty tank conditions. the increased values in full tank condition are very small varying compare to empty tank.
5. The critical response of elevated water tanks does not always occur the same conditions as mentioned above, it may vary also due to depending on the earthquake characteristics

## REFERENCES

1. Kulvendra patel, Wind and Seismic Analysis of Elevated Water tank using Staad Pro, International Research Journal of Engineering and Technology, e-ISSN: 2395-0056, Volume: 05 Issue: 10 | october-2018.
2. Bharat Bhushan Jindal, Dhirendra Singhal, Comparative Study of Design of Water Tank with Reference to IS 3370, Research gate, Conference paper: March 2012.
3. N. Krishna Raju, Advanced Reinforced Concrete Design (IS 456-2000), 2nd Edition 2005, CBS Publishers and Distributors Pvt Ltd.
4. Gaikwad Madhurar V, Prof. Mangulkar Madhuri N , Comparison between Static and Dynamic Analysis of Elevated Water Tank, International Journal of Scientific \& Engineering Research, ISSN 2229-5518, Volume 4, Issue 6, June-2013.
5. Prof. Patel Nikunjr, Prof. Jugal Mistry Analysis of Circular Water Tank Stresses Under Hydrostatic Loading by Using Stadd Pro Software, Indian Journal of Research, ISSN - 2250-1991, Volume : 5 | Issue : 9 | September 2016
6. Hariteja N., Yogesh Kaushik, Rohit Varma M., Sachin Sharma and Sameer Pathania, Seismic Assessment of Elevated Circular Water Tank, International Journal of Engineering Technology Science and Research, ISSN 2394 - 3386 Volume 3, Issue 5 May 2016.
7. S.S. Bhavikatti, Advanced R.C.C Design Volume II 2nd Edition, New Age International Pvt Ltd.
8. IS 1893:2014(Part 2) Draft Copy Criteria for earthquake resistant design of structures, Part 2 Liquid retaining tanks.
9. IS 3370:1967, IS 3370:2009 Indian Standard concrete structures for storage of liquids.
10. IS 456:2000 Indian standard plain and reinforced concrete
