

COMPARATIVE STUDY OF COLD FORMED SECTION STEEL FOOT OVER BRIDGE UNDER DIFFERENT CONFIGURATION

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Abstract - This paper deals with the comparative study of Foot over Bridge under different configuration. In this study three types of bridges are considered and from that the best configuration is selected. In this study the bridges are analyzed and designed by using two software's i.e., Tekla structures and Staad-Pro. Results are compared for all and the best suitable foot over bridge is selected under the criteria of light-weight and economical design.

Key Words: Foot over bridge, Steel Truss, Cable-Stayed, Cold Formed Steel, Staad-Pro, Tekla Structures

1.INTRODUCTION:

A foot over bridge which is also called as pedestrian bridge, pedestrian overpass bridge, or pedestrian overcrossing bridge) is a bridge designed for pedestrians and in some cases cyclists, animal traffic, and horse riders, instead of vehicular traffic.

Foot bridges are used for landscaping and aesthetic purpose which visually links two different areas and for connecting purpose for roads. In many developed countries, footbridges provides both functional and beautiful works of art and sculpture. For rural communities in the developing world, a footbridge may be a community's only access to medical clinics, schools and markets, which would otherwise be unreachable when rivers are too high to cross. Simple suspension bridge designs have been developed to be sustainable and easily constructible in such rural areas using only local materials and labor.

Sometimes the foot bridge which connects two buildings is called as skyway. Bridges which provide for both pedestrians and cyclists are often referred to as green bridges and form an important part of sustainable transport movement towards more sustainable cities. Foot over bridges is often situated to allow pedestrians to cross water or railways in areas where there are no nearby roads to necessitate a road bridge. They are also located across roads for safely pedestrian crossings without disturbance in traffic. The latter is a type of pedestrian separation structure, examples of which. are particularly found near schools, to help prevent children running in front of moving cars. Foot bridge can be built with readily available materials and basic tools.

1.1 Need of Foot-over Bridge

A pedestrian bridge, also called a footbridge, is simply a bridge, can either be above ground or water which is designed for pedestrian traffic. Foot bridges are constructed for pedestrian to cross road from one side to another in a safe way. The pedestrian bridges are also built over railway tracks, rivers, parking lots, canyons and other areas were walking even impossible. Pedestrian bridges are constructed to help pedestrians cross from one side to the other without having to dash across at the risk of being knocked down by speeding vehicles, and also help to ease traffic

1.2 Objectives:

- To Compare the design behaviour for different configuration of members by studying the structural analysis and to provide manual design, software design and comparison between them for economical and weight.
- To design the CFS box section beams and columns using the codal provisions requirements to adopt the economical section.

2. Review of Literature:

They have investigated the performance of cold-formed steel foot over bridge at that city. The main purpose of this paper is to design a bridge which is harmless, more economical and very simple to assemble foot over bridges for walkers. In this paper the analysis of hollow steel box section using STAAD Pro. Software. The thickness of the sheet which is of steel is 2 to 3 mm and yield strength of the steel sheet is 280 *N/mm*2 the cold-formed steel box section of the foot over bridge is constructed because of bending operation simple and low cost. Design of cold-formed steel box-section columns and beams are used EUROCODES EN 1993 and done manually. The authors conclude that the cold-formed box section will reduce the dead weight of the structure and provides high strength and durability

3. Methodology:

The aim of the study is to compare the behavior of the structural members under different configuration for foot over bridges. In this study results will be compared by using two softwares i.e., STAAD-Pro and TEKLA structure for simple steel foot over bridge and cable-stayed foot over bridge and then by using Cold Formed Steel sections as a structural member. Also Studying load behavior patterns, and from all of this the most efficient, applicable, lightweight and economical bridge will be finalized.

3.1 Survey:

• Pedestrian Traffic

The number of persons crossing the road is 2,500 daily. The times that the pedestrian counts were made greatly impacted the outcome. It can be seen the greatest pedestrian flow across the highway occurred between the hours of 7am to 8am, 2pm to 3pm and 5pm to 6pm because most of the students leaved in this time from institute.

Vehicular traffic-

Due to heavy loaded vehicles travelling continuously on highway, the pedestrians crossing the highway has major risk of accidents

3.2 Location and size of pedestrian bridge:



Fig -1: Location of Foot-Over Bridge

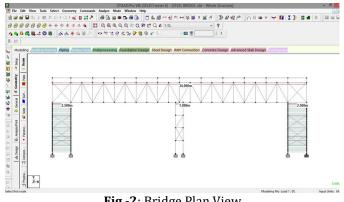
• Design dimensions

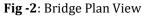
After conducting the survey in front of road following dimensions were decided with the help of measuring tape.

- a. Width of service road=7.2m
- b. Width of divider near service road=5m
- c. Width of highway=26.72 m
- d. Width of divider between two lanes of highway=1 m
- e. Width of staircase=3m
- f. Height=6.5m above G.L.
- g. Total effective span=58m
- Following are the design options for considered location:
- 1. Cold formed section truss foot bridge in Staad-Pro.
- 2. Cold formed section truss foot bridge in Tekla Str.

3.3 Geometry and Modelling of Foot-over bridge:

With the help of the measurements taken during site survey, the following information is required for the modelling of the basic bridge structural geometry





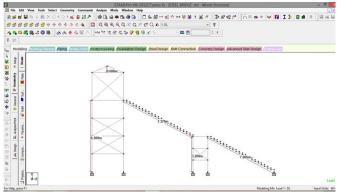


Fig -3: Bridge Elevation View

• Design Loading:

The loads were applied to bridge structures in STAAD-PRO and Tekla Structures are

1) Dead load: Dead load means the weight of the material used in the construction of the bridge. Dead load considerations are taken from IS 875 - (Part-1):1987.

Dead load = Self-weight of all members

2. 2)Pedestrian live load: The live load acting on the deck of the bridge. Live load means the weight of the pedestrians walking over the deck. The live load considerations are taken from the provisions with all the considerations from IS 875 – (Part-2):1987.

live load= $2KN/m^2$

3)Wind load: Since this is an open structure, the wind load is taken into consideration. Wind load is considered according to Indian standards of code IS875(Part-3): 1987.IS code reference clauses and tables.

3.3.1 Modelling of Cold formed section truss foot bridge in Staad Pro.

Firstly, as shown in Fig-4 Modelling of the foot-over bridge is done in Staad-Pro as per dimensions described.

Then section property, loads and load combinations are given to the structure.



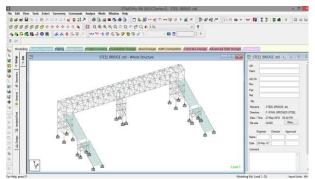


Fig -4: Modeling of Cold Formed Steel Bridge in Staad-Pro

| SR | MEMBER NAME | SECTION | MATERIAL |
|-----|-----------------|---------------|----------|
| NO. | | | |
| 1. | Deck Slab | Plate | Steel |
| | | Thickness=8mm | |
| 2. | Staircase Plate | Plate | Steel |
| | | Thickness=5mm | |
| 3. | Main Girder | TUB 132x132x5 | Steel |
| 4. | Cross Girder | TUB 113x113x5 | Steel |
| 5. | Column Bracing | TUB 113x113x5 | Steel |
| 6. | Main Column | TUB 100x100x6 | Steel |
| 7. | Mid- Landing | TUB 100x100x6 | Steel |
| | Column | | |
| 8. | Members Below | TUB 96x48x5 | Steel |
| | Tread | | |
| 9. | Stringer Beam | ISMC200 | Steel |
| 10. | Vertical | TUB 113x113x5 | Steel |
| | Member of Top | | |
| | Truss | | |
| 11. | All Members of | TUB 113x113x5 | Steel |
| | Bottom Truss | | |
| 12. | Cross Member | TUB 40x40x3.2 | Steel |
| | of Bottom Truss | | |
| 13. | Incline Member | TUB 80x40x5 | Steel |
| | of Bottom Truss | | |
| 14. | Roof Truss | TUB 40x40x3.2 | Steel |
| 15. | Riser and Tread | ISMC 100 | Steel |

 Table -1: Sectional Properties for Bridge in Staad-Pro

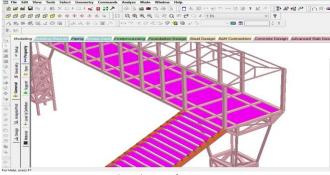


Fig -5: Render View

3.3.1 Modelling of Cold formed section truss foot bridge in Tekla Structures.

Firstly, as shown in Fig-6 Modelling of the foot-over bridge is done in Tekla as per dimensions described.

Then section property, loads and load combinations are given to the structure.

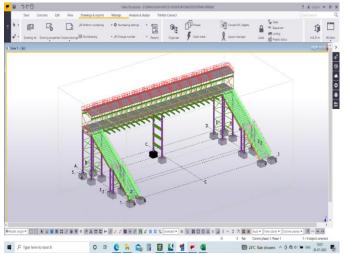


Fig -6: Modeling of Cold Formed Steel Bridge in Tekla Str.

| Table -2: Sectional | Properties for Bridge | in Tekla Struc. |
|---------------------|-----------------------|-----------------|
| | | |

| SR | MEMBER | SECTION | MATERIAL | |
|-----|-----------------|--------------------|----------|--|
| NO. | NAME | | | |
| 1. | Deck Slab | Plate | Steel | |
| | | Thickness=8mm | | |
| 2. | Staircase Plate | Plate | Steel | |
| | | Thickness=5mm | | |
| 3. | Main Girder | TUB 150x150x5 | Steel | |
| 4. | Cross Girder | TUB 132x132x5 | Steel | |
| 5. | Column | TUB 89x89x4.5 | Steel | |
| | Bracing | | | |
| 6. | Main Column | ISMB 300 | Steel | |
| 7. | Mid- Landing | TUB 89X89X4.5 | Steel | |
| | Column | | | |
| 8. | Members | TUB 96x48x5 | Steel | |
| | Below Tread | | | |
| 9. | Stringer Beam | ISMC200 | Steel | |
| 10. | Vertical | TUB 100X50X4.7X7.5 | Steel | |
| | Member of | | | |
| | Top Truss | | | |
| 11. | All Members | TUB 132x132x5 | Steel | |
| | of Bottom | | | |
| | Truss | | | |
| 12. | Cross Member | TUB 132x132x5 | Steel | |
| | of Bottom | | | |
| 10 | Truss | | | |
| 13. | Incline | TUB 45X45X4.5 | Steel | |
| | Member of | | | |
| 14 | Bottom Truss | | Charal | |
| 14. | Roof Truss | TUB 50X50X6 | Steel | |
| 15. | Riser and | ISMC 100 | Steel | |
| | Tread | | | |



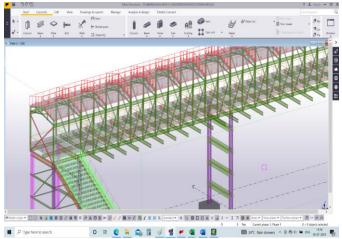


Fig -7: Render View

4. Results and Discussions:

 The result for Case 1 i.e., Cold formed steel Section Bridge in Staad-Pro is as follows and deflection can be studied for various members in this case under post processing of STAAD Pro. Weight of bridge is 242.191 KN. Following figure shows maximum deflection of a node in truss. Allowable deflection = Span/250 = 136mm whereas resultant deflection is 20.969 mm. Therefore, it is safe.

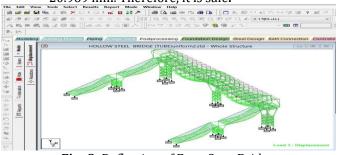


Fig -8: Deflection of Foot-Over Bridge

| | | | Horizontal | Vertical | Horizontal | Resultant | Rotational | | |
|--------|------|--------------|------------|----------|----------------------|-----------|------------|-----------|-----------|
| | Node | L/C | X | Y mm | Z mm | mm | rX rad | rY rad | rZ rad |
| Max X | 977 | 12 1.5(DL+W | 7.942 | -5.778 | -0.09 <mark>4</mark> | 9.822 | -0.001 | 0.000 | -0.001 |
| Min X | 963 | 3 1.5(DL+LL) | -5.373 | -0.377 | -0.352 | 5.397 | -0.000 | -0.000 | 0.001 |
| Max Y | 233 | 10 1.2(DL+LL | 0.545 | 1.418 | 5.257 | 5.473 | 0.001 | -0.000 | 0.001 |
| Min Y | 83 | 3 1.5(DL+LL) | 0.460 | -20.949 | -0.520 | 20.960 | -0.000 | -0.000 | 0.000 |
| Max Z | 469 | 14 1.5(DL+W | -0.504 | -2.614 | 8.830 | 9.222 | -0.000 | 0.000 | 0.000 |
| Min Z | 309 | 3 1.5(DL+LL) | 0.816 | -4.135 | -2.061 | 4.692 | 0.003 | 0.000 | 0.000 |
| Max rX | 222 | 3 1.5(DL+LL) | 0.988 | -7.726 | 0.786 | 7.828 | 0.004 | -0.000 | 0.000 |
| Min rX | 60 | 3 1.5(DL+LL) | 1.016 | -7.681 | -0.986 | 7.810 | -0.004 | 0.000 | 0.000 |
| Max rY | 1026 | 3 1.5(DL+LL) | -1.337 | -7.811 | -0.114 | 7.926 | 0.004 | 0.001 | 0.000 |
| Min rY | 990 | 3 1.5(DL+LL) | -1.386 | -7.810 | -0.283 | 7.937 | -0.004 | -0.001 | 0.000 |
| Max rZ | 874 | 3 1.5(DL+LL) | 1.199 | -3.718 | -0.124 | 3.909 | 0.000 | -0.000 | 0.004 |
| Min rZ | 906 | 3 1.5(DL+LL) | 2.640 | -4.210 | -0.250 | 4.976 | -0.000 | -0.000 | -0.004 |
| Max Rs | 80 | 3 1.5(DL+LL) | 0.932 | -20.947 | -0.217 | 20.969 | -0.000 | -0.000 | 0.000 |

Fig -9: Summary Sheet (a)

 The result for Case 2 i.e., Cold formed steel Section Bridge in Tekla software is as follows and deflection can be studied for various members in this case. In case of steel truss foot over bridge which analyzed in Tekla structures, the weight of bridge is 255.71 kN. Allowable deflection = Span/250 = 122mm whereas resultant deflection is 18.269 mm. Therefore, it is safe.

| Nos | Part. Pos | Profile | Qua ntity | Length/ mm | Area / m2 | Weigh t.Net / t | |
|-----|--------------|-------------------------------|--------------|---------------|-----------|-----------------------|-------|
| 6 | 1 | ISMB300 | 6 | 6,668 | 43.39 | 1.708 | 1.84 |
| 95 | 4 | 2350*50 | 95 | 300 | 152.47 | 25.19 | 26.29 |
| 6 | 6 | ISMB300 | 6 | 1,300 | 8.46 | 0.339 | 0.359 |
| 4 | 16 | ISMB300 | 4 | 7,379 | 32.01 | 1.262 | 1.358 |
| 2 | 17 | ISMB300 | 2 | 3,300 | 7.16 | 0.292 | 0.304 |
| 2 | 19 | ISMB300 | 2 | 3,300 | 7.16 | 0.292 | 0.304 |
| 14 | M1 | ISMB300 | 14 | 6,950 | 105.52 | 4.357 | 4.476 |
| 8 | M2 | ISMB300 | 8 | 3,650 | 31.67 | 1.308 | 1.343 |
| 8 | MЗ | RHS80*40* 4 | 8 | 2,492 | 4.71 | 0.14 | 0.14 |
| 8 | M4 | RHS80*40* 4 | 8 | 2,909 | 5.49 | 0.164 | 0.164 |
| 5 | M5 | SHS150*5 | 5 | 34,340 | 103.04 | 3.909 | 3.909 |
| 22 | M6 | SHS132*5.4 | 22 | 5,700 | 66.27 | 2.692 | 2.692 |
| 1 | M7 | ISMB200 | 1 | 5,700 | 4.24 | 0.134 | 0.138 |
| 54 | M8 | B_BUILT10 0*50*4.7*7. 5 | 54 | 2,708 | 86.03 | 1.774 | 1.774 |
| 4 | M9 | SHS45+4.5 | 4 | 4,066 | 2.93 | 0.093 | 0.093 |
| 46 | M10 | SHS45+4.5 | 46 | 2,962 | 24.56 | 0.78 | 0.78 |
| 1 | M11 | SHS45+4.5 | 1 | 3,189 | 0.57 | 0.018 | 0.018 |
| 1 | M12 | SHS45*4.5 | 1 | 3,177 | 0.57 | 0.018 | 0.018 |
| 30 | M13 | ISMC100 | 30 | 300 | 3.37 | 0.083 | 0.086 |
| 30 | M14 | ISMC75 | 30 | 1,677 | 14.41 | 0.343 | 0.359 |
| 30 | M15 | ISMC75 | 30 | 1,710 | 14.7 | 0.35 | 0.367 |
| 30 | M16 | ISA50X50X 6 | 30 | 450 | 2.63 | 0.06 | 0.06 |

Fig -10: Summary Sheet (b)

| | - | Total | 550 | 13,32,983 | 1,293.67 | 200 | 257.6 |
|----|------|----------------|-----|-----------|----------|-------|-------|
| 1 | pl11 | 80*1720 | 1 | 2,882 | 10.61 | 3.101 | 3.114 |
| 1 | pl10 | 80*1732 | 1 | 2,893 | 10.67 | 3.119 | 3.147 |
| 1 | p19 | 80*1720 | 1 | 2,591 | 9.59 | 2.796 | 2.799 |
| 1 | pl8 | 80*1720 | 1 | 2,594 | 9.59 | 2.796 | 2.802 |
| 11 | pl7 | 80*1720 | 11 | 2,420 | 98.4 | 28.62 | 28.77 |
| 1 | pl6 | 80*1720 | 1 | 2,598 | 9.58 | 2.793 | 2.807 |
| 11 | p15 | 80*1732 | 11 | 2,420 | 98.95 | 28.78 | 28.97 |
| 1 | pl4 | 80*1720 | 1 | 2,598 | 9.59 | 2.794 | 2.807 |
| 1 | pl3 | 150*3045 | 1 | 3,300 | 22 | 11.83 | 11.83 |
| 12 | pl2 | 150*2400 | 12 | 3,300 | 210.6 | 111.9 | 111.9 |
| 1 | pl1 | 150*2495 | 1 | 3,300 | 18.21 | 9.695 | 9.695 |
| 8 | M25 | SHS89+4.5 | 8 | 3,140 | 8.96 | 0.3 | 0.3 |
| 2 | M24 | ISMC100 | 2 | 3,300 | 2.47 | 0.061 | 0.063 |
| 14 | M23 | SHS89*4.5 | 14 | 3,300 | 16.47 | 0.552 | 0.552 |
| 1 | M22 | ISA50X50X 5 | 1 | 2,909 | 0.57 | 0.011 | 0.011 |
| 15 | M21 | SHS89*4.5 | 15 | 2,909 | 15.56 | 0.521 | 0.521 |
| 16 | M20 | SHS89*4.5 | 16 | 2,492 | 14.22 | 0.476 | 0.476 |
| 15 | M19 | ISA50X50X 6 | 15 | 870 | 2.54 | 0.058 | 0.058 |
| 15 | M18 | ISA50X50X 6 | 15 | 865 | 2.53 | 0.057 | 0.058 |
| 15 | M17 | ISA50X50X 6 | 15 | 418 | 1.22 | 0.028 | 0.028 |

Fig -11: Summary Sheet (b) continued.

Impact Factor value: 7.529



5. CONCLUSIONS:

Coming at solution for problem of pedestrian traffic to cross road without interrupting vehicular traffic and trying various cases of different type of foot over bridges. The components are designed for the maximum safety and the adaptability of the structure to future changes has also been given due consideration. Weight of Cold-formed Steel foot over bridge was low comparatively hence, it is concluded that Cold form Steel section bridge is light-weight and therefore it is economical also.

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